By Hand

Our Ref: S3088/LTTM/23/005Lg

27 February 2024

Secretary, Town Planning Board 15/F, North Point Government Offices 333 Java Road North Point Hong Kong



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Dear Sir/Madam,

Proposed Rezoning from "Residential (Group B)1" Zone to "Residential (Group B)4" Zone for Medium-Density Housing Development to Include a Footpath for Public Use at Various Lots and Adjacent Government Land in DD130, Lam Tei, Tuen Mun - S12A Amendment of Plan Application – TPB Ref.: Y/TM-LTYY/11 Further Information No. 1

Reference is made to the captioned S12A Application submitted to the Town Planning Board ("TPB") on 22 January 2024.

The Permanent Secretary for Transport and Logistics gazetted the PWP Item Nos. B764CL and B861CL Site Formation and Infrastructure Works for Public Housing Developments at San Hing Road and Hong Po Road, Tuen Mun (Road Works) on 26 January 2024. The Applicant has reviewed the gazette plan and noticed that there are changes in some of the road layout and traffic improvement measures near the Site. In order to reflect the latest road layout and traffic arrangement in the vicinity, we hereby submit a revised Traffic Impact Assessment (enclosed in **Annex A**) for consideration by the Transport Department and TPB.

Should you have any queries in relation to the attached, please do not hesitate to contact the undersigned at the second or our Ms Anson YING at the second of the second second

Thank you for your kind attention.

Yours faithfully For and on behalf of KTA PLANNING LIMITED

Gladys Ng

Encl. (4 hard copies)

cc. the Applicant & Team KT/GN/AY/vy







Revised Traffic Impact Assessment

Traffic Impact Assessment

Final Report February 2024

Prepared by: CKM Asia Limited

Prepared for: Wing Mau Tea House Limited

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1.0 INTRODUCTION

Background

- 1.1 The subject site is located in D.D.130, Lam Tei, Tuen Mun (the "Subject Site"). At present, the Subject Site is unoccupied, and access to the Subject Site via an existing unnamed road which is connected to Ng Lau Road. The location of the Subject Site is shown in **Figure 1.1**.
- 1.2 A Section 12A planning application for the minor relaxation of the maximum plot ratio restriction to 2.5 for residential use at the Subject Site was approved by the Town Planning Board (TPB ref: Y/TM-LTYY/9) on 24th September 2021 (the "Approved Scheme"). This Section 12A planning application is for minor relaxation of the maximum plot ratio restriction for residential use at the Subject Site from the approved 2.5 to 5.0 (the "Proposed Development").
- 1.3 Against this background, CKM Asia Limited, a traffic and transportation planning consultancy firm, was commissioned by the Owner to conduct a traffic impact assessment in support of the Proposed Development. This report presents the findings and recommendations of the traffic impact assessment for the Proposed Development.

Structure of Report

1.4 The report is structured as follows:

Chapter One	 Gives the background of the project;
Chapter Two	- Describes the existing situation;
Chapter Three	- Presents the Proposed Development;
Chapter Four	- Describes the traffic impact analysis; and
Chapter Five	- Gives the overall conclusion.

2.0 EXISTING SITUATION

The Subject Site

2.1 The Subject Site is bounded by the Light Rail Transit ("LRT") and the Tuen Ma Line to the East, and a nullah to the West. Access to the Subject Site is from the south and is via a bridge over the nullah. The Access Road is connected to Ng Lau Road.

The Road Network

- 2.2 Ng Lau Road is a single carriageway 2-lane 2-way local distributor which connects with the Lam Tei Interchange to the south and Castle Peak Road Lam Tei underneath the Kong Sham Western Highway. It provides access to villages, e.g., San Hing Tsuen, Tuen Tsz Wai, and Tsing Chuen Wai.
- 2.3 Lam Tei Interchange connects Tsing Lun Road, Hong Po Road, Ng Lau Road, Castle Peak Road – Lam Tei, Yuen Long Highway and Tuen Mun Road. It is the main access for traffic accessing the Subject Site and strategic routes.

Existing Traffic Flows

- 2.4 To quantify the traffic flows in the vicinity of the Subject Site, manual classified counts were conducted on Tuesday, 18th April 2023, Wednesday, 19th April 2023 and Wednesday, 26th April 2023 during the AM and PM peak at the following junctions:
 - J1: Unnamed Road/ Access Road;
 - J2: Ng Lau Road/ Unnamed Road;
 - J3: Ng Lau Road / Lam Tei Interchange;
 - J4: Tsing Lun Road/ Hong Po Road/ Lam Tei Interchange;
 - J5: Lam Tei Interchange;
 - J6: Lam Tei Interchange/ Castle Peak Road Lam Tei;
 - J7: Tsing Lun Road/ Tsz Tin Road;
 - J8: San Hing Road / Ng Lau Road (Southern);
 - J9: San Hing Road / Ng Lau Road (Northern);
 - J10: T-junction at San Hing Road;
 - J11: Ng Lau Road / Castle Peak Road Lam Tei; and
 - J12: Hong Po Road / Yan Tin Estate Access Road.
- 2.5 The locations of these junctions and the area of influence (the "AOI") are shown in **Figure 2.1** and the layouts are shown in **Figures 2.2 2.13** respectively.
- 2.6 The traffic counts are classified by vehicle type to enable traffic flows in passenger car units ("pcu") to be calculated. The AM and PM peak hours identified from the surveys are found to be between 0800 0900 hours and 1700 1800 hours respectively. The existing AM and PM peak hour traffic flows in pcu/hour are presented in Figure 2.14.

Existing Junction Performance

2.7 The existing junction performance of the surveyed junctions are calculated based on the existing traffic flows, and the analysis was undertaken using the methods outlined in Volume 2 of the Transport Planning and Design Manual ("TPDM"). The results are summarised in **Table 2.1** and the detailed calculations are found in **Appendix A**.

TABLE 2.1	EXISTING JUNCTION	PERFORMANCE
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Ref.	Junction	Type of Junction (Parameter)	AM Peak	PM Peak
J1	Unnamed Road/ Access Road	Priority (DFC)	0.000	0.000
J2	Ng Lau Road/ Unnamed Road	Priority (DFC)	0.033	0.030
J3	Ng Lau Road/ Lam Tei Interchange	Signal (RC)	104%	95%
J4	Tsing Lun Road/ Hong Po Road/ Lam Tei Interchange	RA (DFC)	0.501	0.548
J5	Lam Tei Interchange	RA (DFC)	0.480	0.453
J6	Lam Tei Interchange/ Castle Peak Road – Lam Tei	Signal (RC)	129%	223%
J7	Tsing Lun Road/ Tsz Tin Road	Signal (RC)	67%	111%
J8	San Hing Road/ Ng Lau Road (Southern)	Priority (DFC)	0.057	0.037
J9	San Hing Road/ Ng Lau Road (Northern)	Priority (DFC)	0.223	0.496
J10	T-junction at San Hing Road	Priority (DFC)	0.006	0.002
J11	Ng Lau Road / Castle Peak Road – Lam Tei	Signal (RC)	140%	133%
J12	Hong Po Road / Yan Tin Estate Access Road	Priority (DFC)	0.060	0.011

Note: RC – reserve capacity; DFC – design flow/capacity ratio, RA – Roundabout

2.8 The above results indicate that the surveyed junctions currently operate with capacities during the AM and PM peak hours.

Link Operational Performance

2.9 The link operational performance of the surveyed road links are calculated based on the existing traffic flows, and the analysis was undertaken using the methods outlined in Volume 2 of the Transport Planning and Design Manual ("TPDM"). The results are summarised in **Table 2.2**.

Ref	Link		Adjusted Design		Traffic Demand		V/C Ratio	
			Flow (/eh/hr)	(veh/hr)			
			AM Peak	PM Peak	AM Peak	PM Peak	AM Peak	PM Peak
L1	Castle Peak Road – Lam	NB	2,604	2,520	763	1,314	0.29	0.52
	Tei	SB	2,604	2,604	1,444	763	0.55	0.29
L2	Castle Peak Road –	NB	2,800	2,800	385	419	0.14	0.15
	Lingnan	SB	2,604	2,800	625	450	0.24	0.16
L3	Yuen Long Highway	NB	4,700	4,700	4,462	4,810	0.95	1.02
		SB	4,700	4,371	3,615	3,962	0.77	0.91
L4	Tuen Mun Road	NB	4,700	4,700	4,821	5,833	1.03	1.24
		SB	4,371	4,371	4,360	3,933	1.00	0.90
L5	San Hing Road	2-way	744	800	43	29	0.06	0.04
L6	Ng Lau Road (north of J9)	2-way	744	800	215	344	0.29	0.43
L7	Ng Lau Road (south of J2)	2-way	744	800	249	371	0.33	0.46
L8	Lam Tei Interchange	EB	2,604	2,800	917	638	0.35	0.23
	(between J3 and J5)	WB	2,800	2,800	1,011	1,100	0.36	0.39
L9	Tsing Lun Road	NB	1,767	1,767	448	371	0.25	0.21
		SB	1,900	1,900	775	685	0.41	0.36
NB – northbound SB – southbound EB – eastbound WB – westbound								

 TABLE 2.2
 EXISTING LINK CAPACITY ASSESSMENT

2.10 The above results show that the assessed road links operate with sufficient capacity, except for Tuen Mun Road northbound, which is operating with V/C ratios at 1.24 during PM peak hours.

Public Transport Facilities

2.11 The Subject Site is located close to public transport services, including franchised buses and public light buses and these operate within 400 metres or some 8-minutes' walk away. Details of these public transport services are presented in Table 2.3. The location and major pedestrian routes of these public transport services are shown in Figure 2.15.

TABLE 2.3ROAD-BASEDPUBLICTRANSPORTSERVICESOPERATINGCLOSE TO THE SUBJECT SITE

Route	Routing	Frequency (min)
CTB 50 ⁽¹⁾	Tuen Mun (Ching Tin and Wo Tin) \rightarrow Tsim Sha Tsui (Kowloon Station) ^(A)	4 per day
	Tsim Sha Tsui (Kowloon Station) \rightarrow Tuen Mun (Ching Tin and Wo Tin) ^(B)	4 per day
CTB 55 ⁽¹⁾	Tuen Mun (Ching Tin and Wo Tin) \rightarrow Kwun Tong Ferry Pier ^(A)	4 per day
	Kwun Tong Ferry Pier \rightarrow Tuen Mun (Ching Tin and Wo Tin) ^(B)	4 per day
CTB 56 ⁽¹⁾	Tuen Mun (Ching Tin and Wo Tin) \rightarrow Sheung Shui (Tin Ping Estate) ^(A)	4 per day
	Sheung Shui (Tin Ping Estate) \rightarrow Tuen Mun (Ching Tin and Wo Tin) ^(B)	4 per day
CTB 56A ⁽¹⁾	Tuen Mun (Ching Tin and Wo Tin) \rightarrow Queen's Hill Fanling (via: Sheung Shui Station) ^(C)	3 per day
	Queen's Hill Fanling (via: Sheung Shui Station) \rightarrow Tuen Mun (Ching Tin and Wo Tin) ^(C)	2 per day
CTB 950 ⁽¹⁾	Tuen Mun (Ching Tin and Wo Tin) \rightarrow Exhibition Centre Station ^(A)	1 per day
	Exhibition Centre Station \rightarrow Tuen Mun (Ching Tin and Wo Tin) ^(B)	1 per day
CTB 955 ⁽¹⁾	Tuen Mun (Ching Tin and Wo Tin) \rightarrow Sai Wan Ho ^(A)	1 per day
	Sai Wan Ho \rightarrow Tuen Mun (Ching Tin and Wo Tin) ^(B)	1 per day
СТВ ВЗА	Shan King Estate - Shenzhen Bay Port	30 - 60
CTB N969 ^(D)	Tin Shui Wai Town Centre - Causeway Bay (Moreton Terrace)	20 - 45
KMB 53	Yoho Mall (Yuen Long) - Tsuen Wan (Nina Tower)	25 - 35
KMB 63X	Hung Shui Kiu (Hung Fuk Estate) - Jordan (West Kowloon Station)	12 - 30
KMB 67M	Tuen Mun (Siu Hong Court) - Kwai Fong Station	5 - 20
KMB 67X	Tuen Mun (Siu Hong Court) - Mong Kok East Station	6 - 25
KMB 68A	Long Ping Estate - Tsing Yi Station	8 - 25
KMB 258A ⁽¹⁾	Hung Shui Kiu (Hung Fuk Estate) → Lam Tin Station	2 per day
KMB 258P ⁽²⁾	Hung Shui Kiu (Hung Fuk Estate) - Lam Tin Station	12 - 30
KMB 261P	Tuen Mun (Siu Hong Court) \rightarrow Sheung Shui (Tin Ping) ^{(2)(A)}	2 - 3 per day
	Sheung Shui (Tin Ping) → Tuen Mun (Siu Hong Court) (1)(B)	1 per day
KMB 267X ⁽¹⁾	Tuen Mun (Siu Hong Court) \rightarrow Lam Tin Station ^(A)	2 per day
	Lam Tin Station → Tuen Mun (Siu Hong Court) ^(B)	2 per day
KMB 960A ⁽¹⁾	Central \rightarrow Hung Shui Kiu (Hung Fuk Estate) ^(B)	1 per day
KMB 960C ⁽¹⁾	Tuen Mun (Fu Tai Estate) \rightarrow Causeway Bay (Victoria Park) ^(A)	2 per day
	Causeway Bay (Victoria Park) → Tuen Mun (Fu Tai Estate) ^(B)	1 per day
KMB 960P	Hung Shui Kiu (Hung Yuen Road) → Causeway Bay (Victoria Park)	10 - 35
	Causeway Bay (Victoria Park) \rightarrow Hung Shui Kiu (Hung Yuen Road) ^{(1)(B)}	1 per day
KMB 960X ⁽¹⁾	Hung Shui Kiu (Hung Yuen Road) → Quarry Bay (King's Road) ^(A)	8 per day
	Quarry Bay (King's Road) \rightarrow Hung Shui Kiu (Hung Yuen Road) ^(B)	8 per day
KMB N260 ^(D)	Tuen Mun Pier Head - Mei Foo	20 - 25
LWB A34	Hung Shui Kiu (Hung Yuen Road) - Airport (Ground Transportation Centre)	20 - 60
LWB E33P	Siu Hong Station (South) - Airport (Ground Transportation Centre)	12 - 45
LWB NA33 ^(D)	Tuen Mun (Fu Tai Estate) \rightarrow Cathay Pacific City	3 per day
	Cathay Pacific City → Tuen Mun (Fu Tai Estate)	5 per day
LWB NA37 ^(D)	Tin Shui Wai Town Centre → Cathay Pacific City	5 per day
	Cathay Pacific City → Tin Shui Wai Town Centre	6 per day
NLB B2	Yuen Long MTR Station - Shenzhen Bay Port	25 - 40
GMB 42	Tsing Chuen Wai - Tuen Mun Town Centre	13 – 15
GMB 606S ^(D)	Yuen Long (Fung Cheung Rd) - Tsim Sha Tsui East	6 - 13
KMB – Kowloor	Motor Bus IWB – Long Win Bus CTB – CityBus GMB – Green	Minibus

NLB – New Lantao Bus

Note: ⁽¹⁾ Monday to Friday. (Except public holidays) ^(A) AM peak only ^(B) PM peak only ⁽²⁾ Monday to Saturday (Except public holidays) ^(C) AM and PM peak only ^(D) Overnight service

Survey on Road-based Public Transport Services Located in the Vicinity

Road-based Public Transport

2.12 Survey on road-based public transport services listed in **Table 2.3** was conducted during the AM and PM peak periods on Thursday, 18th January 2024 at the bus stops near the subject site. The survey locations are shown in **Figure 2.15**. The survey results are summarized in **Table 2.4** and the detailed information are shown in **Appendix B**.

TABLE 2.4 OCCUPANCY OF EXISTING ROAD-BASED PUBLIC TRANSPORT SERVICES OPERATING NEAR THE SUBJECT SITE

Direction	AM Peak				PM Peak	'eak	
	No. of Passenger C		Occupancy	No. of Passenger		Occupancy	
	Capacity [a]	Occupied [b]	[c]=[b]/[a]	Capacity [d]	Occupied [e]	[f]=[e]/[d]	
Outbound – To other	5,057	3,588	71%	2,398	813	34%	
districts							
Inbound – From other	2,297	823	36%	2,915	1,823	63%	
districts							

2.13 The above results indicate that the surveyed road-based public transport services currently operate with spare capacities during the AM and PM peak hours.

Rail-based Public Transport

2.14 Based on the information obtained from the Legislative Council, the operational performance for MTR Tuen Ma Line in 2022 is summarized in **Table 2.5**.

TABLE 2.5OPERATIONAL PERFORMANCE OF MTR TUEN MA LINE

Item	Parameters
Maximum carrying capacity when train frequency is	70,000 passengers / hour
maximized [a]	
Existing carrying capacity [b]	58,800 passengers / hour ⁽¹⁾
Current Patronage [c]	34,500 passengers / hour ⁽²⁾
Current Loading [c]/[b] {Critical Link}	59% {Tsuen Wan West to Mei Foo}
Loading compared with maximum carrying capacity [c]/[a]	50%

Source: Reply Serial No. TLB168 for Question Serial No. 1237, Controlling Officer's Reply, Examination of Estimates of Expenditure 2023-24. Finance Committee. Legislative Council. 14 April 2023.

https://www.legco.gov.hk/yr2023/english/fc/fc/w_q/tlb-e.pdf>

- ⁽¹⁾ According to the reply, existing train frequency has not yet increased to the maximum level as permitted by the signaling system.
- ⁽²⁾ According to the reply, in view of the impact of COVID-2019, patronage shown is based on those months in 2022 when the pandemic situation was relatively eased.
- 2.15 **Table 2.5** shows that the MTR Tuen Ma Line operates at 59% of its current capacity, or 50% of its maximum carrying capacity during the peak hour.

Existing Footpath Level-Of-Service

2.16 To quantify the existing pedestrian flows, pedestrian counts were conducted during the AM and PM peak periods on Thursday, 18th January 2024 at footpaths located in the vicinity of Proposed Development, and the observed peak 15-minute pedestrian flows are shown in **Figure 2.16**.

2.17 The Level-Of-Service ("LOS") of a pedestrian footpath depends on its width and number of pedestrians using the facility. Description of the LOS at walkway is obtained from Volume 6 of the TPDM and is presented in **Table 2.6**.

TABLE 2.6	DESCRIPTION OF PEDESTRIAN FOOTPATH LOS
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LOS	Flow Rate	Description
	(ped/min/m)	
A	≤ 16	Pedestrians basically move in desired paths without altering their movements in response to
		other pedestrians. Walking speeds are freely selected, and conflicts between pedestrians are
		unlikely.
В	16 – 23	Sufficient space is provided for pedestrians to freely select their walking speeds, to bypass
		other pedestrians and to avoid crossing conflicts with others. At this level, pedestrians begin
		to be aware of other pedestrians and to respond to their presence in the selection of walking
		paths.
С	23 - 33	Sufficient space is available to select normal walking speeds and to bypass other pedestrians
		primarily in unidirectional stream. Where reverse direction or crossing movement exist,
		minor conflicts will occur, and speed and volume will be somewhat lower.
D	33 – 49	Freedom to select individual walking speeds and bypass other pedestrians is restricted.
		Where crossing or reverse-flow movements exist, the probability of conflicts is high and its
		avoidance requires changes of speeds and position. The LOS provides reasonable fluid flow;
		however considerable friction and interactions between pedestrians are likely to occur.
E	49 – 75	Virtually, all pedestrians would have their normal walking speeds restricted. At the lower
		range of this LOS, forward movement is possible only by shuffling. Space is insufficient to
		pass over slower pedestrians. Cross- and reverse-movement are possible only with extreme
		difficulties. Design volumes approach the limit of walking capacity with resulting stoppages
		and interruptions to flow.
F	> 75	Walking speeds are severely restricted. Forward progress is made only by shuffling. There
		are frequent and unavoidable conflicts with other pedestrians. Cross- and reverse-
		movements are virtually impossible. Flow is sporadic and unstable. Space is more
		characteristics of queued pedestrians than of moving pedestrian streams.
Sourc	e: Volume 6	Chapter 10 of TPDM

2.18 The observed peak 15-minute pedestrian flows LOS assessment is presented in **Table 2.7**.

TABLE 2.7EXISTING LOS ASSESSMENT

Location	Clear Width ⁽¹⁾ [Effective Width] (m)	Peak Period	Flow (ped/ 15 min)	Flow rate (ped/min/m)	LOS
P1. Footpath on the footbridge connected to	2.0[1.5]	AM	50	2.2	A
Ng Lau Road		PM	33	1.5	A
P2. Footpath between Lam Tei LRT stop and	2.5[1.5]	AM	105	4.7	A
bus stop at Castle Peak Road – Lam Tei		PM	70	3.1	Α
P3. Footbridge over Castle Peak Road – Lam	2.5[1.5]	AM	72	3.2	A
Tei		PM	35	1.6	A

⁽¹⁾ The width excludes railing and obstructions.

2.19 The above results indicate that the surveyed footpaths currently operate with LOS A during the AM and PM peak. As stated in the TPDM, LOS A to C is considered as an acceptable level of service: "In general, LOS C is desirable for most design at streets with dominant 'living' pedestrian activities".

3.0 THE PROPOSED DEVELOPMENT

Key Parameters

3.1 The Proposed Development key parameters are presented in **Table 3.1**.

TABLE 3.1KEY PARAMETERS

	Item	Proposed Development		
Development Site Area		About 8,896 m ²		
Domestic Plot Ratio		5.0		
Domestic GFA		44,480 m ²		
Flat Mix	Flat Size $\leq 40m^2$	1,110		
(GFA)	$40m^2$ < Flat Size \leq 70m ²	275		
Total num	ber of Flats	1,385		

Provision of Internal Transport Facilities

3.2 The internal transport facilities for the Proposed Development are provided in accordance with the recommendations of the Hong Kong Planning Standards and Guidelines ("HKPSG") and are presented in **Table 3.2**.

TABLE 3.2PROVISIONOFINTERNALTRANSPORTFACILITIESFORPROPOSEDDEVELOPMENT

Facility	HKPSG Recommendation		Provision
Car	For Residents:		
Parking	$\overline{Parking Requirement} = GPS \times R1 \times R2 \times R3$	222 nos . @ 5.0m (L) x	
Space	Global Parking Standard (GPS): 1 car parki	ng space per 4 - 7	2.5m (W) x 2.4m (H)
-	flats	= HKPSG maximum	
	Demand Adjustment Ratio (R1): 0.5 for flat	size \leq 40 m ² GFA	
	1.2 for flat	size 40 – 70 m ²	
	GFA		
	Accessibility Adjustment Ratio(R2): 1.0 outside	500m-radius of rail	
	station		
	Development Intensity Adjustment Ratio (R3):	1.0 for Plot Ratio	
	2.0 – 5.0		
	For 1,100 flats with flat size less than 40 m ² GF	A	
	Minimum: $(1,110 / 7 \times 0.5 \times 1.0 \times 1.0) = 79.3$, say 80 nos.	
	Maximum: $(1,110 / 4 \times 0.5 \times 1.0 \times 1.0) = 138$.	8, say 139 nos.	
	For 275 flats with flat size 40 – 70 m ² GFA		
	Minimum: $(275 / 7 \times 1.2 \times 1.0 \times 1.0) = 47.2$, say 48 nos.	
	Maximum: $(275 / 4 \times 1.2 \times 1.0 \times 1.0) = 82.5$, say 83 nos.	
	Total		
	Minimum = 80 + 48 = 128 nos.		
	Maximum = 139 + 83 = 222 nos.		
	For Visitors:		35
	Visitor car parking for private residential develo	opments with more	25 nos . (22 nos. @
	block in addition to the recommendations, or a	o visitor spaces per	$2.4m(H) + 2.5m(VV) \times 10^{-1}$
	the Authority	is determined by	$5.0m(1) \times 3.5m(M) \times 3.5m(M)$
	the Admonty.		2.4m(H) for person with
	For 5 blocks: $5 \times 5 \text{ pos} = 25 \text{ pos}$		disabilities)
	1013 blocks. 3 x 5 1103. 25 1103.	= HKPSG maximum	
	Total Car Parking Space:		247 nos. (including 3
	Minimum = 128 + 25 = 153 nos.		accessible car parking
	Maximum = 222 + 25 = 247 nos.		spaces)
	Note: For total no. of car parking space in lot $= 151$	– 250 nos., the	
	Building (planning) regulation /2 require provision of parking spaces	of 3 accessible car	
	parking spaces		

TABLE 3.2PROVISIONOFINTERNALTRANSPORTFACILITIESFORPROPOSEDDEVELOPMENT (CONT'D)

Facility	HKPSG Recommendation	Provision
Motorcycle	For Residential Uses:	18 nos. @ 2.4m (L) x
Parking	TD Comment: 1 motorcycle parking space shall be	1.0m (W) x Min. 2.4m
Space	provided for every 81 flats	(H)
		= fulfil TD comment,
	For 1,385 flats:	ОК
	1,385 / 81 = 17.1, say 18 nos.	
Goods	For Residential Uses:	5 nos. @ 11.0m (L) x
Vehicle	Minimum of 1 loading / unloading bay for goods vehicles	3.5m (W) x Min. 4.7m
Loading/	within the site for every 800 flats or part thereof, subject to a	(H)
Unloading	minimum of 1 bay for each housing block or as determined	= HKPSG minimum,
Bay	by the Authority.	ОК
	For 5 blocks, each block less than 800 flats: <u>5 no.</u>	
Bicycle	For Residential Uses:	93 по. @ 1.8m (L) x
Parking	Within 0.5 – 2km to rail station, 1 space per 15 flats with flat	0.8m (W) x Min. 2.4m
Spaces	size <70m ²	(H)
	$= 1,385 \div 15$	= comply HKPSG,
	= 93 nos.	ОК

3.3 **Table 3.2** shows that the internal transport facilities provided comply with the recommendations of the HKPSG. The master layout plan of the Proposed Development is shown in **Figure 3.1**.

Planned Road Works near the Proposed Development

3.4 The existing access road and unnamed road connecting the Proposed Development with Ng Lau Road is planned to be improved, to provide a 7.3m-wide road carriageway, a 2m-wide footpath and a 2m-wide cycle track (the "Planned Road Works"). The Planned Road Works to be implemented by the Owner as part of the Approved Scheme and is found in **Appendix C**.

Swept Path Analysis

3.5 The CAD-based swept path analysis programme, Autodesk Vehicle Tracking, was used to check the ease of manoeuvring of vehicles within the Proposed Development, and the swept path analysis drawings are found in **Appendix D**. Vehicles are found to have no manoeuvring problems.

4.0 TRAFFIC IMPACT

Design Year

4.1 The Proposed Development is expected to be completed in 2030, and the design year adopted for the traffic assessment is, whichever later of the 2: (i) at least 3 years after the planned completion of the development, i.e., 2033, or (ii) 5 years from the date of this application, i.e., 2028. Therefore, Year 2033 is adopted for junction capacity analysis.

Traffic Forecasting

4.2 Year 2033 peak hour traffic flows for the junction capacity analysis is produced (i) with reference to the 2019-based BDTM NTW1 (the "BDTM"); (ii) estimated growth from 2031 to 2033; (iii) expected traffic generation by the planned / committed developments in the vicinity; and (iv) expected traffic generation by the 2 cases, i.e., Approved Scheme and Proposed Development.

Modelling and Validation

- 4.3 The BDTM provides traffic forecasts for the years 2026 and 2031 and these have taken into account the planned developments, changes to the strategic road network, population growth, etc. Therefore, The BDTM is used as the basis to produce the traffic flow for this TIA.
- 4.4 The BDTM is validated, and the validation meets criteria found in the "BDTM Study". Nevertheless, the traffic network and zone in the vicinity of the Proposed Development were further reviewed to ensure the traffic model is upto-date and the modelled flow can be adopted. The modelling and validation methodology include, but not limited to, the following:
 - The road links and junctions were checked and updated to ensure that any recent change in the existing road network is considered and missing road links or junction does not exist.
 - The schedules of public transport services such as franchised bus and green/ red minibus were also checked to ensure that the updated routings and headway information are adopted.
 - The zone and centroid connectors were reviewed to ensure that the traffic zones generate/ attract traffic at appropriate locations.
 - The traffic flows produced by BDTM at the surveyed junctions were reviewed with reference to the observed traffic flows.
 - The validation methodology is same as that adopted in the BDTM. All count locations were reviewed and checked using the GEH statistic (a modified chi squared test to provide a statistic for both the magnitude of the difference and the percentage difference between modelled and observed flows). The GEH statistic is defined by:

$$\sqrt{rac{(V_2-V_1)^2}{rac{1}{2}(V_2+V_1)}}$$

where V_1 and V_2 are the observed and modelled flows.

4.5 The validation criteria adopted are found in **Table 4.1**.

TABLE 4.1VALIDATION CRITERIA

Locations	Target
Traffic flows at all count locations	85% return a GEH statistic of 5 or less
	100% return a GEH statistic of 10 or less

Estimated Traffic Growth Rate from 2031 to 2033

4.6 Reference is made to the "Hong Kong Population Projections 2022 – 2046" published by Census and Statistics Department, and the information is presented in **Table 4.2**.

 TABLE 4.2
 HONG KONG POPULATION PROJECTIONS FROM CENSUS

 AND STATISTICS DEPARTMENT

Year	Population in Hong Kong (thousands)
2031	7,820.2
2033	7,903.6
Average Annual Growth (2031 – 2033)	0.53%

4.7 **Table 4.2** shows that the annual population growth between 2031 and 2033 is 0.53%, and is adopted for estimated traffic growth rate from 2031 to 2033.

Additional Planned/ Committed Developments near the Subject Site

4.8 The planned/ committed developments near the Subject Site not included in the BDTM but have been incorporated to produce the future year traffic flows are listed in **Table 4.3** and the locations are presented in **Figure 4.1**.

TABLE 4.3	THE ADDITIONAL	PLANNED / COMMITTED	DEVELOPMENTS
	NEAR THE SUBJECT	SITE	

Ref. No.	Development	Intake Year	Land Use	GFA (m²)	No. of Flat (no.)	Average Flat Size (m²)	No.
Tue	n Mun Area 54 (1)				-	-	
Α	Site 1 &1A	2022	PRH		4,232		
	Wo Tin Estate		Retail	2,420			
			SWF	1,060			
			Kindergarten				1 no.
В	Site 2	2017	PRH		4,688		
	Yan Tin Estate		Retail	4,250			
			SWF	3,600			
С	Site 3 & 4 (East)	2022	PRH		5,183		
	Ching Tin Estate		Retail	3,130			
			SWF	1,810			
			Kindergarten				1 no.
D	Site 3 & 4 (West)	2025	Private Housing		4,600		
	Novo Land		Retail	5,000			
E	Site 4A (East and	2026	Light Public Housing		5,620		
	West) ⁽²⁾						
G	Site 4A (South)	2028	PRH		1,475		
			Kindergarten				1 no.
Н	Site 5	2028	SSF		1,020		
			SWF	1,300			

TABLE 4.3THE ADDITIONAL PLANNED / COMMITTED DEVELOPMENTS
NEAR THE SUBJECT SITE (CONT'D)

Ref. No.	Development	Intake Year	Land Use	GFA (m ²)	No. of Flat (no.)	Average Flat Size (m ²)	No.
Dev	elopment at San Hir	ng Road a	nd Hong Po Road, Tuen	Mun ⁽³⁾			
I	San Hing Road	2030	PRH / SSF		9,400		
	Site	-	Primary School				1 nos.
		2033	Kindergarten				2 nos.
			SWF	N/A			
J	San Hing Road	2030	PRH / SSF		1,500		
	Site Extension		Retail	5,000 ⁽⁴⁾			
		2033	Sport Centre				1 no.
K	Ho Pong Road	2030	PRH / SSF	-	9,500		
	Site	-	Retail	5,000 ⁽⁴⁾			
		2033	Kindergarten				2 no.
			SWF	N/A			
Oth	er Planning Applicat	tions Nea	rby ⁽⁵⁾				
L	A/TM-LTYY/ 426	2026	Private Housing		184	31	
Μ	Y/TM-LTYY/ 10		Private Housing		288	40	
Ν	A/TM-LTYY/ 301		NTEH ⁽³⁾		1	195	
0	A/TM-LTYY/ 335		NTEH ⁽³⁾		1	195	
Р	A/TM-LTYY/ 336		NTEH ⁽³⁾		1	195	
Q	A/TM-LTYY/ 370		NTEH ⁽³⁾		1	195	
R	A/TM-LTYY/ 371		NTEH ⁽³⁾		1	195	
S	A/TM-LTYY/ 372		NTEH ⁽³⁾		1	195	

PRH – Public Rental Housing SSF – Subsidised Sale Flats NTEH – New Territories Exempted House SWF – Social Welfare Facilities

(2) extracted from Legislative Council Panel on Housing discussion paper CB(1)1123/2023(02) on December 2023

- (3) extracted from Tuen Mun District Council discussion paper TMDC 19/2023 on September 2023
- (4) No information on area for retail uses is found in public domain, assumed 5,000 m² GFA of retail
- (5) extracted from Planning Statement of Approved Planning Applications

Planned Road Improvement Works Nearby

4.9 The planned road improvement works at assessed junctions are presented below.

Development at San Hing Road and Hong Po Road

4.10 Some road improvement works are planned under the "Site Formation and Infrastructure Works for Public Housing Developments at San Hing Road and Hong Po Road, Tuen Mun and Choi Shun Street, Sheung Shui – Investigation, Design and Construction" (Agreement No. CE 39/2021 (CE)) by Civil Engineering and Development Department ("CEDD"), and these are summarized in **Table 4.4**. The road improvement works are found in **Appendix E**.

TABLE 4.4PLANNED ROAD IMPROVEMENT WORKS UNDER AGREEMENT
NO. CE 39/2021 (CE) BY CEDD

Ref	Brief Description of the Improvement
J3	Provide 2 left-turn lanes at Ng Lau Road southbound
	Provide 1 left-turn lane at Lam Tei Interchange eastbound
J4	Provide exclusive left-turn lane from Hong Po Road southbound
	Modify the entry lanes from Lam Tei Interchange westbound
J6	Provide 2 right-turn lanes and 1 shared lane for right turn and straight ahead at Castle Peak
	Road – Lam Tei southbound
J7	Provide a channelized island at Tsz Tin Road eastbound*
J10	Widened to provide 2 lane 2-way single carriageway at minor road
J12	Provide signalised cross junction

⁽¹⁾ extracted from TIA of Approved Planning Applications A/TM/500 and A/TM/583

4.11 The improvement work described in **Table 4.4** will be completed gradually before 2030 – 2033, i.e., the intake of public housing of San Hing Road site, and San Hing Road site extension and Hong Po Road site (Note: These are items I, J and K in **Table 4.3**). These improvement works are adopted for the Year 2033 junction capacity analysis.

Hung Shiu Kiu New Development Area

4.12 Road improvement work is planned at Ng Lau Road / Castle Peak Road – Lam Tei (J11) under the "Hung Shui Kiu/Ha Tsuen New Development Area Package A Works for Second Phase Development - Design and Construction" (Agreement No. CE 01/2020 (CE)) by Civil Engineering and Development Department ("CEDD"). The layout of road improvement at J11 is presented in **Appendix E**.

Net Increase in Traffic Generation between the Approved Scheme and the Proposed Development

4.13 To estimate the traffic generation of the Proposed Development, reference is made to the TPDM. However, the smallest flat size in the TPDM is 60m² GFA, which is substantially larger than the Proposed Development average flat size of only 32m² GFA. Hence, the estimated traffic generation is conservative, i.e., on the high-side. The adopted trip generation rates and the estimated AM and PM peak hour traffic generation are presented in **Table 4.5**.

TABLE 4.5ADOPTEDTRIPRATESANDTRAFFICGENERATIONFORPROPOSEDDEVELOPMENT

Proposed Development	AM Peak		PM Peak			
	Generation	Attraction	Generation	Attraction		
Trip Rates(pcu/ flat/ hr)						
Residential Use with average 60m ² GFA	0.0718	0.0425	0.0286	0.0370		
Traffic Generations (pcu/ hr)						
1,385 flats with average flat about 32m ² GFA	100	<u>59</u>	<u>40</u>	52		
	159		92			

4.14 The traffic generation of Approved Scheme found in the approved traffic impact assessment is presented in **Table 4.6**.

TABLE 4.6ADOPTED TRAFFIC GENERATION FOR APPROVED SCHEME

Scheme	AM Peak		PM Peak	
	Generation	Attraction	Generation	Attraction
Approved Scheme	37	22	<u>18</u>	23
	5	9	4	1

4.15 The net increase in traffic generation between the Approved Scheme and the Proposed Development is presented in **Table 4.7**.

Scheme	Traffic Generation (pcu/ hr)					
	AM Peak PM Peak			Peak		
	Generation	Attraction	Generation	Attraction		
Proposed Development (from Table 4.4) [a]	100	59	40	52		
Approved Scheme (from Table 4.5) [b]	37	22	18	23		
Net Increase [a] – [b]:	+ 63	+ 37	+22	+ 29		
	+ 100		+51			

TABLE 4.7NET INCREASE IN TRAFFIC GENERATION

4.16 **Table 4.7** shows that the Proposed Development is expected to generate 100 and 51 additional pcu (2-way) in AM and PM peak respectively.

Year 2033 Traffic Flows

4.17 Year 2033 traffic flows for the following cases are derived:

Year 2033 Without	_	Traffic flows derived with reference to 2031 NTW1 BDTM
Proposed Development		+ estimated traffic growth between 2031 and 2033 +
[A]		estimated traffic generation of the planned / committed
		developments after 2019

Year 2033 With Approved = [A] + estimated traffic generation for Approved Scheme Scheme [B]

Year 2033 With Proposed = [B] + net increase in traffic generation by Proposed Development [C] Development

4.18 Year 2033 peak hour traffic flows for the above three cases are shown in Figures4.2 – 4.4 respectively.

Year 2033 Junction Capacity Analysis

4.19 Year 2033 junction capacity analysis for the three cases are summarised in Table4.7 and detailed calculations are found in the Appendix A.

Ref	Junction	Type of Junction (Parameter)	2033 V Prop Develo	Vithout osed opment	2033 Appr Scho	2033 With Approved Scheme		With osed pment
			AM Peak	PM Peak	AM Peak	PM Peak	AM Peak	PM Peak
J1	Unnamed Road/ Access Road	Priority (DFC)	0.055	0.049	0.056	0.050	0.059	0.051
J2	Ng Lau Road/ Unnamed Road	Priority (DFC)	0.052	0.046	0.128	0.079	0.266	0.125
J3	Ng Lau Road/ Lam Tei Interchange	Signal (RC)	42%	48%	39%	46%	35%	44%
J4	Tsing Lun Road/ Hong Po Road/ Lam Tei Interchange	RA (DFC)	0.694	0.659	0.703	0.666	0.719	0.675
J5	Lam Tei Interchange	RA (DFC)	0.797	0.668	0.806	0.677	0.822	0.691
J6	Lam Tei Interchange/ Castle Peak Road – Lam Tei	Signal (RC)	26%	49%	26%	49%	26%	49%
J7	Tsing Lun Road/ Tsz Tin Road	Signal (RC)	23%	61%	22%	60%	21%	59%
J8	San Hing Road/ Ng Lau Road (Southern)	Priority (DFC)	0.091	0.055	0.094	0.060	0.100	0.065
J9	San Hing Road/ Ng Lau Road (Northern)	Priority (DFC)	0.198	0.448	0.198	0.448	0.198	0.448
J10	T-junction at San Hing Road	Priority (DFC)	0.058	0.071	0.059	0.074	0.063	0.078
J11	Ng Lau Road / Castle Peak Road – Lam Tei	Signal (RC)	16%	15%	16%	15%	16%	15%
J12	San Hing Road / Hong Po Road	Signal (RC)	55%	96%	121%	197%	116%	195%

TABLE 4.8YEAR 2033 JUNCTION PERFORMANCE

Note: RC - reserve capacity; RA - Roundabout, DFC - design flow/capacity ratio

4.20 **Table 4.8** shows that the Proposed Development has negligible traffic impact to the road junctions analysed.

Year 2033 Link Performance

4.21 The 2033 link performances are assessed and the results are shown in **Table 4.9**.

			Adju	usted	Year 2033 Traffic Demand (veh/hr)					Year 2033 V/C Ratio						
			Desig	n Flow	Wit	hout	W	ith	W	ith	Wit	hout	W	ith	W	ith
Def	Link		(veh	hr)	Prop	osed	Appr	oved	Prop	osed	Prop	osed	Appr	oved	Prop	osed
Kei	LINK				Develo	pment	Sch	eme	Develo	pment	Develo	opment	Sch	eme	Develo	pment
			AM	PM	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM	AM	PM
			Peak	Peak	Peak	Peak	Peak	Peak	Peak	Peak	Peak	Peak	Peak	Peak	Peak	Peak
1.1	Castle Peak	NB	2,604	2,604	864	1,434	864	1,434	865	1,434	0.33	0.55	0.33	0.55	0.33	0.55
LI	Road – Lam Tei	SB	2,604	2,604	1,810	1,238	1,810	1,238	1,812	1,240	0.70	0.48	0.70	0.48	0.70	0.48
12	Castle Peak	NB	2,800	2,800	501	581	501	581	502	582	0.18	0.21	0.18	0.21	0.18	0.21
LZ	Road – Lingnan	SB	2,604	2,800	808	562	810	562	812	563	0.31	0.20	0.31	0.20	0.31	0.20
1.2	Yuen Long	NB	4,700	4,700	5,677	6,373	5,683	6,377	5,694	6,384	1.21	1.36	1.21	1.36	1.21	1.36
LJ	Highway	SB	4,700	4,700	5,099	5,190	5,104	5,195	5,111	5,198	1.09	1.10	1.09	1.11	1.09	1.11
1.4	Tuon Mun Bood	NB	4,700	4,700	6,103	7,445	6,115	7,456	6,131	7,469	1.30	1.59	1.30	1.59	1.30	1.59
L4	Tuen Mun Koad	SB	4,700	4,700	6,135	5,542	6,147	5,548	6,163	5,557	1.31	1.18	1.31	1.18	1.31	1.18
L5	San Hing Road	2-way	800	800	174	80	187	84	212	91	0.22	0.10	0.23	0.11	0.27	0.11
L6	Ng Lau Road (north of J9)	2-way	744	800	251	370	251	370	251	370	0.34	0.46	0.34	0.46	0.34	0.46
L7	Ng Lau Road (south of J2)	2-way	800	800	411	436	467	497	512	508	0.51	0.55	0.58	0.62	0.64	0.64
	Lam Tei Interchange	EB	2,800	2,800	1,731	1,180	1,752	1,191	1,784	1,208	0.62	0.42	0.63	0.43	0.64	0.43
Lð	(between J3 and J5)	WB	2,800	2,800	1,779	1,705	1,798	1,722	1,826	1,742	0.64	0.61	0.64	0.62	0.65	0.62
10	Tring Lup Bood	NB	1,900	1,900	823	628	842	657	827	633	0.43	0.33	0.44	0.35	0.44	0.33
19	TSINg Lun Koad	SB	1,900	1,900	1,275	929	1,282	932	1,296	933	0.67	0.49	0.67	0.49	0.68	0.49
NB –	northbound	SB ·	– south	bound	E	B – eas	tbound		WB – w	/estbou	nd					

TABLE 4.9 YEAR 2033 LINK CAPACITY ASSESSMENT

4.22 The above results show that the assessed road links operate with sufficient capacity, except for Yuen Long Highway (L3) and Tuen Mun Road (L4), both which operate with V/C ratios at 1.2 or above during the AM and PM peak hours in Year 2033. In view that there are no changes on the V/C ratios to L3 and L4 for cases without Proposed Development, with Approved Scheme and with Proposed Development, it can be concluded that the traffic generated due to the Proposed Development have negligible impact. With the planned strategic road improvement works, both Yuen Long Highway and Tuen Mun Road are expected to operate with sufficient capacity in Year 2033.

5.0 IMPACT TO PUBLIC TRANSPORT SERVICES

2033 Public Transport Occupancy Forecasting

5.1 2033 peak hour public transport occupancy is estimated based on (i) public transport demand growth from 2024 to 2033; and (ii) public transport demand generated by the Proposed Development and planned / committed developments in the vicinity.

Annual Public Transport Demand Growth Rate between 2024 – 2033

- 5.2 To establish the local public transport demand growth rate from 2024 to 2033, reference is made to several sources of information including:
 - 2024 2029: "Projections of Population Distribution 2021 2029" published by Planning Department
 - 2029 2033: "Hong Kong Population Projections" from the Census and Statistics Department
- 5.3 The "Projections of Population Distribution 2021 2029" has Tertiary Planning Units ("TPU"), i.e., the local area population projections up to 2025, and reference is made to 5 relevant TPUs, which are presented in **Table 5.1**.

Year		Total			
	423 & 428	425	441	442	
2024	225,800	70,200	16,600	7,100	319,700
2025	227,100	71,300	16,300	7,100	321,800
Average Annual Growth 2024 to 2025	0.58%	1.57%	-1.81%	0.00%	0.66%

TABLE 5.1 POPULATION PROJECTIONS OF THE 5 TPUS

- 5.4 **Table 5.1** shows that the average annual population growth between 2024 and 2025 is 0.66%.
- 5.5 Between 2025 and 2029, reference is made to the population growth of Tuen Mun New Town, and population projections are presented in **Table 5.2**.

TABLE 5.2	TUEN MUN NEW TO	WN POPULATION PROJECTIONS

Year	Tuen Mun New Town Population
2025	557,400
2029	575,400
Average Annual Growth 2025 to 2029	0.80%

- 5.6 **Table 5.2** shows that the average annual population growth in the Tuen Mun New Town between 2025 and 2029 is 0.8%.
- 5.7 Beyond 2029, reference is made to the *"Hong Kong Population Projections"* from the Census and Statistics Department, which is presented in **Table 5.3**.

TABLE 5.3 HONG KONG POPULATION PROJECTIONS FROM CENSUS AND STATISTICS DEPARTMENT

Year	Hong Kong Resident Population ('000)
2029	7,731.1
2033	7,903.6
Average Annual Growth 2029 to 2033	0.55 %

- 5.8 **Table 5.3** shows that the average annual population growth in Hong Kong between 2029 2033 is 0.55%.
- 5.9 Based on the above, the annual growth factors adopted are 0.66% from 2024 to 2025, 0.8% between 2025 and 2029, and 0.55% between 2029 and 2033.

Estimated Peak Hour Mechanised Trip Generation of Subject Site

5.10 The mechanised trip generation of the Subject Site is estimated with reference to Travel Characteristic Survey 2011 and are presented in **Table 5.4**.

TABLE 5.4 ESTIMATED PEAK HOUR MECHANISED TRIP GENERATION OF THE SUBJECT SITE

Parameter	Calculation	Unit	Approved Scheme	Proposed Development
No. of Flats	А	flats	307	1,385
Average domestic household size in Tuen Mun ⁽¹⁾	В	persons/ flat	2.6	2.6
Population	$C = B \times A$	persons	812	966
Average Daily Mechanised Trips ⁽²⁾	D	trips/ persons/ day	1.83	1.83
Peak hour factor of Daily Mechanised Trips ⁽³⁾	E	N/A	12%	12%
Estimated Peak Hour Mechanised Trip	D =	persons/ hr	176	<mark>7</mark> 91
Generation	A x B x C			

⁽¹⁾ Extracted from Census and Statistic Department website

⁽²⁾ From Table 3.3, Travel Characteristics Survey 2011 Final Report

⁽³⁾ From Para. 3.3.7, Travel Characteristics Survey 2011 Final Report

Estimated Public Transport Demand Generated by the Subject Site

5.11 The transport mode of the Subject Site in the vicinity is assumed with reference to *"Travel Characteristic Survey 2011"* and is presented in **Table 5.4**, and the estimated public transport demand is calculated and shown in **Table 5.5**.

TABLE 5.5TRANSPORT MODE OF THE SUBJECT SITE

Т	ransport Mode	Ratio ⁽¹⁾		The Subject Site						
			Proposed		Net Increase in Passenger					
				Development [b]	Demand [C] = [b] – [a]					
Public	Rail-based [a]	44% ⁽²⁾	77	348	+271					
Transport	Road-based [b]	38% ⁽²⁾	67	301	+234					
	Sub-total $[c] = [a] + [b]$	<u>82%</u>	<u>144</u>	<u>649</u>	+ 505					
Private Car / Taxi [d]		18%	32	142	+ 110					
Total	[e]=[c]+[d]	100%	176	791	+615					

⁽¹⁾ From Table 3.6, Travel Characteristics Survey 2011 Final Report

⁽²⁾ Adjusted based on local public transport provision near the subject site

5.12 **Table 5.5** shows that compared with the Approved Scheme, the Proposed Development is expected to generate additional public transport demand of 505 passengers per hour (2-way) during both AM and PM peak hours. The roadbased public transport demand generated by the subject site is summarised in **Table 5.6**.

TABLE 5.6 ESTIMATED ROAD-BASED PUBLIC TRANSPORT DEMAND GENERATED BY THE SUBJECT SITE

	Development	Road-bas	sed Public Ti (persons /	ransport Den hour)	nand
		AM Pe	Peak		
		Generation	Attraction	Generation	Attraction
The	Approved Scheme: 307 Flats [a]	61	6	6	61
Subject	Proposed Development: 1,385 Flats [b]	271	30	30	271
Site	Net increase of road-based Passenger	210	24	24	210
	Demand [b] – [a]	+234 (2	-way)	+234 (<mark>2-wa</mark> y)

5.13 **Tables 5.6** shows that compared with the Approved Scheme, the Proposed Development is expected to generate additional road-based public transport demand of 234 passengers per hour (2-way) during both AM and PM peak hours.

Public Transport Demand Generated by Planned / Committed Developments in the Vicinity

5.14 The public transport demand generated by planned / committed developments in the vicinity as presented in **Table 4.3** is considered in the Year 2033 public transport demand. Public transport interchanges are provided for the 2 planned development areas, i.e., "Tuen Mun Area 54" and "Development at San Hing Road and Hong Po Road, Tuen Mun". It is assumed that the public transport services provided would be sufficient to serve the demand generated by these development areas.

2033 Road-based Public Transport Occupancies

- 5.15 Year 2033 road-based public transport occupancies were derived with reference to the (i) observed road-based public transport occupancies in **Table 2.4**; (ii) annual public transport demand growth rate; and (iii) expected road-based public transport demand due to the planned / committed developments between 2024 – 2033 and the subject site.
- 5.16 Year 2033 road-based public transport occupancies were derived as follows:

2033 without the	_	2024 observed occupancy + adopted road-based public					
Proposed Development		transport demand growth from 2024 to					
[A]		2033 + estimated road-based public transport demand					
		due to the planned / committed developments					
2033 with the Approved	_	[A] + estimated road-based public transport demand					
Scheme [B]		due to Approved Scheme					
2033 with the Proposed	_	[B] + net increase in estimated road-based public					
Development [C]		transport demand by Proposed Development					

5.17 The Year 2033 road-based public transport occupancies for the three cases are summarised in **Table 5.7**.

TABLE 5.7 YEAR 2033 ROAD-BASED PUBLIC TRANSPORT OCCUPANCY OPERATING NEARBY DURING PEAK HOURS

Direction	Case		AM Peak	c		PM Peal	0
Direction		No. of	Passenger	- Occupancy	No. of I	Passenger	Occupancy
		Capacity	Occupied	[c]=[b]/[a]	Capacity	Occupied	[f]=[e]/[d]
		[a]	[b]		[d]	[e]	
To other	Without Proposed Development	11,821	4,621	39%	2,776	846	30%
districts	With Approved Scheme	11,821	4,682	40%	2,776	852	31%
	With Proposed Development	11,821	4,892	41%	2,776	876	32%
From	Without Proposed Development	3,181	1,049	33%	9,121	2,031	22%
other	With Approved Scheme	3,181	1,055	33%	9,121	2,092	23%
districts	With Proposed Development	3,181	1,079	34%	9,121	2,302	25%

5.18 **Table 5.7** shows that the road-based public transport demand associated with the Proposed Development has negligible impact.

2033 Rail-Based Public Transport Occupancies

5.19 **Table 5.5** shows that the demand on rail-based public transport services, i.e. MTR Tuen Ma Line, associated with the Proposed Development is no more than 348 passengers during the peak hours. As shown in **Table 2.5**, the MTR Tuen Ma Line has a maximum carrying capacity of 70,000 passenger / hour. Hence, the additional passenger demand is only 0.5% of the maximum carrying capacity [Calculation: 348 ÷ 70,000 = 0.5%], which is negligible on the MTR Tuen Ma Line.

6.0 **PEDESTRIAN IMPACT**

2033 Pedestrian Flow Forecasting

2033 peak 15-minute pedestrian flows are produced by estimating (i) the 6.1 pedestrian growth from 2024 to 2033; and (ii) expected pedestrian generated by the Proposed Development and planned / committed developments in the vicinity.

Annual Pedestrian Growth Rate between 2024 – 2033

Growth rates of 0.66% per annum from 2024 to 2025, 0.8% per annum for the 6.2 period between 2025 and 2029, and 0.55% per annum for the period between 2029 and 2033, are adopted, and reference to these are found in **Paragraphs 5.2 - 5.9**.

Peak 15-minute Pedestrian Generated by Planned / Committed Developments in the Vicinity

Peak 15-minute pedestrian generated by planned / committed developments in 6.3 the vicinity as presented in **Table 4.3** is included in the Year 2033 pedestrian flow.

Peak 15-minute Pedestrian Generation of by the Proposed Development

Based on public transport demand presented in **Table 5.5**, the peak 15-minute 6.4 pedestrian generations of the Subject Site are shown in **Table 6.1**.

	Developments	Pedestrian Generations (ped / 15-minute)							
		AM	Peak	PM Peak					
		GEN	ATT	GEN	ATT				
The	Approved Scheme: 307 Flats [a]	21	2	2	21				
Subject	Proposed Development: 1385 Flats [b]	91	10	10	91				
Site	Net Increase of Pedestrian Generation	+ 70	+8	+8	+70				
	[b] – [a]	+78 (2-way)		+ 78 (2-way)					

TABLE 6.1 PEDESTRIAN GENERATIONS OF THE SUBJECT SITE

GEN – Generation ATT – Attraction

6.5 Tables 6.1 shows that compared with the Approved Scheme, the additional pedestrian generated by the Proposed Development is 78 persons (2-way) during both AM and PM peak 15 minutes.

Year 2033 Pedestrian Flows

- 6.6 Year 2033 pedestrian flows are produced with reference to (i) the observed 2024 pedestrian flows, (ii) annual pedestrian growth rate between 2024 - 2033, (iii) expected pedestrian generation due to the planned / committed developments between 2024 – 2033 and the Subject Site.
- 6.7 Year 2033 pedestrian flows for the footpath analysis were derived as follows:

2033 with

2033 without Proposed	_	2024 observed pedestrian flows + Adopted pedestrian						
Development [A]		growth from 2024 to 2033 + estimated pedestrian due to the planned / committed developments						
2033 with Approved Scheme [B]	=	[A] + pedestrian generation due to Approved Scheme						
2033 with Proposed Development [C]	=	[B] + net increase in pedestrian generation due to Proposed Development						

Year 2033 LOS Analysis

TABLE 6.2

6.8 Year 2033 peak 15-minute pedestrian flows for the three cases are estimated and presented in Figure 6.1 and the corresponding LOS assessment is presented in **Table 6.2.**

Peak 2033 without 2033 with Location Clear

YEAR 2033 LOS ASSESSMENT

		Width ⁽¹⁾	Period	Proposed			Approved Scheme			Proposed Development		
		[Effective		Development								
		Width] (m)		Flow	Flow	LOS	Flow	Flow	LOS	Flow	Flow	LOS
					rate			rate			rate	
P1.	Footpath on the	2.0[1.5]	AM	80	3.6	А	92	4.1	А	131	5.8	A
	footbridge accessing to											
	Ng Lau Road		PM	62	2.8	А	74	3.3	А	113	5.0	A
P2.	Footpath between Lam	2.5[1.5]	AM	130	5.8	Α	136	6.0	А	156	6.9	A
	Tei LRT stop and bus											
	stop at Castle Peak		PM	93	4.1	Α	99	4.4	А	119	5.3	Α
	Road – Lam Tei											
P3.	Footbridge over Castle	2.5[1.5]	AM	79	3.5	Α	85	3.8	А	105	4.7	Α
	Peak Road – Lam Tei											
			PM	40	1.8	Α	46	2.0	А	66	2.9	Α

Note: Flows in pedestrian / 15 minutes flow rates in pedestrian / 15 minutes / meter ⁽¹⁾ The width excludes railing and obstructions.

6.9 The results in **Table 6.2** show that the assessed footpaths operate with LOS A, i.e., have sufficient capacity to accommodate the expected pedestrian growth and additional pedestrian generated due to Proposed Development.

7.0 SUMMARY

- 7.1 The Subject Site is located in D.D.130, Lam Tei, Tuen Mun. At present, the Subject Site is unoccupied, and access to the Subject Site is via an existing unnamed road which is connected to Ng Lau Road.
- 7.2 Manual classified counts were conducted at junctions which are located in the vicinity in order to establish the existing traffic flows during AM Peak and PM peak hours.
- 7.3 The internal transport facilities provided comply with recommendations of the HKPSG and comments from Transport Department.
- 7.4 Year 2033 peak hour traffic flows for the junction capacity analysis is produced (i) with reference to the BDTM; (ii) estimated growth from 2031 to 2033; (iii) expected traffic generation by the planned / committed developments in the vicinity; and (iv) expected traffic generation by the 2 cases, i.e., Approved Scheme and Proposed Development.
- 7.5 Compared to the Approved Scheme, the Proposed Development will generate only 100 and 51 additional pcu (2-way) in AM peak and PM peak respectively.
- 7.6 The assessment of the nearby public transport services found that the Proposed Development has negligible impact. The assessment of footpaths found that the Proposed Development has negligible impact.
- 7.7 This TIA concluded that compared with the Approved Scheme, the traffic generated by the Proposed Development is negligible. The Proposed Development is acceptable from traffic engineering terms.

Figures



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\JOB\J7250-J7299\J7265\2024 01_R6\Fig 2 1 RevG dwg



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Project Title PROPOSED REZONING FROM "RESIDENTIAL (GROUP B)1" ZONE TO "RESIDENTIAL (GROUP B)4" ZONE FOR MEDIUM-DENSITY HOUSING DEVELOPMENT TO	J7265 2.13 Scale in A4 1 : 500
INCLUDE A FOOTPATH FOR PUBLIC USE AT VARIOUS LOTS AND ADJACENT GOVERNMENT	Designed by Drawn by Checked by Revision Date
LAND IN DUISU, LAM IEI, IUEN MUN (APPLICATION NO. Y/TM-LTYY/11) Figure Title	$\begin{array}{c c c c c c c c c c c c c c c c c c c $
LAYOUT OF JUNCTION OF	Traffic and Transportation Planning Consultants
HONG PO ROAD / YAN TIN ESTATE ACCESS ROAD	21st Floor, Methodist House, 36 Hennessy Road, Wan Chai, Hong Kong
	1ei : (852) 2520 5990 Fax : (852) 2528 6343 Email : mail@ckmasia.com.hk

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HEJECT	SROAD A A A A A A A A A A A A A	
	Development at San Hing Road and Hong Po Road, Tuen Mun	
	I San Hing Road Site	
	K Ho Pong Road Site	
(///Sic # } 3	Other Planning Applications Nearby	
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	M Y/TM-LTYY/ 10	d VO
	N A/TM-LTYY/ 301	> 0 1
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and	P A/TM-LTYY/ 336	202 T
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	S A/TM-LTYY/ 372	2/1/66
L.1 Drawn by WSW Date 20 Fl	Revision CKM Asia Limited Traffic and Transportation Planning Consultants K C 21st Floor, Methodist House, 36 Hennessy Road Wan Chai, Hong Kong Tel : (852) 2520 5990 FB 2024	1 \JOB/J/25U-J/25









Appendix A – Junction Capacity Analysis

















Junction:	Ng Lau Ro	oad / Lam Tei	Intercha	ange											Job Nu	mber:	J7265
Scenario:	Existing C	ondition		<u> </u>												P.	9
Design Year:	2023	Design	ed By:				_	Checke	d By:					Date:	21	I Feb 20	124
	Approach	I	Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
Lam Tei Intercha	ange EB	SA+LT	A1		3 65	30.0	Gradiem	23	(pcu/m)	(pcu/m)	0 273		60	(pcu/m)	(pcu/m)	0 212	
		SA	A2	1	3.65	00.0			2120	579	0.273		00	2120	450	0.212	
							1										
Lam Tei Intercha	ange WB	SA+LT	A3	1	3.65	15.0		0	1980	563	0.284	0.284	0	1980	626	0.316	0.316
		SA	A4	1	3.65				2120	602	0.284			2120	670	0.316	
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Ng Lau Road SB	3	LT	B1	2	4.00	12.0	<u> </u> '	100	1791	203	0.113	0.113	100	1791	180	0.100	0.100
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							1										
pedestrian ph	ase		C _(P)	1		min c	rossing	time =	8	sec (GM +	8	sec F	GM =	16	sec	
			D _(P)	2		min c	rossing	time =	8	sec (GM +	9	sec F	GM =	17	sec	
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AM Traffic Flow (pcu/h	ır)		Ν	PM Traffic	Flow (pcu/hr))			N	S = 1940 +	+ 100 (W-3.	25) S =	= 2080 + 10	0 (W-3.25)	Note:		
	203	3					180 I			SM = S / (1 + 1.5 f/r)	SM =	(S - 230) / ((1 + 1.5 f/r)			
t ¹²	24	→			t ²⁴⁴	ţ	L,		1		AM	Peak	PM	Peak			
	• 989	•			_	614		•			1 + 2		1+2	<u> </u>			
		1165	ļ					1296	l	Sum y	0.398		0.417				
		0	ļ					0	l	L (s)	8		8				
			ļ						l	C (s)	82		82]		
			I						l	practical y	0.812		0.812				
<u> </u>				<u> </u>						R.C. (%)	104%		95%				
1		2				3				4				5			
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AM G	=	I/G = 5	G =		I/G =	5	G =		I/G =		G =		I/G =		G =		
G	-	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G	=	I/G = 5	G =		I/G =	5	G =		I/G =		G =		I/G =		G =		
G	-	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

Signal Junction Analysis

						Signal J	unction A	nalysis									
Junction:	Ng Lau	Road / Lam Tei	Intercha	ange										-	Job Nu	mber:	J7265
Scenario:	Without	t Development														Ρ.	10
Design Year:	2033	Designe	ed By:				_	Checke	d By:					Date:	2′	Feb 20)24
						1				AM Daala					DM Daak		
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	Flow	y value	Critical y	Turning %	Sat. Flow	Flow	y value	Critical y
Lam Tei Intercha	inge EB	SA	Δ2	1	4 00		Gradient		(pcu/m)	815	0.378			2155	(pcu/iii)	0 235	
		SA	A3	1	4 00				2155	814	0.378			2155	506	0.235	
Lam Tei Intercha	inge WB	SA+LT	B1	1	4.00	15.0		0	2015	942	0.467	0.467	0	2015	929	0.461	0.461
		SA	B2	1	4.00				2155	1007	0.467			2155	993	0.461	
Ng Lau Road SB		LT	C1	2	5.50	10.0		100	1883	199	0.106	0.106	100	1883	162	0.086	0.086
		LT	C2	2	5.00	15.0		100	2050	216	0.105		100	2050	176	0.086	
pedestrian pha	ase		D _(P)	1		min c	rossing	time =	5	sec	GM +	13	sec F	GM =	18	sec	
			E _(P)	2		min c	rossing	time =	10	sec	GM +	12	sec F	GM =	22	sec	
AM Traffic Flow (pcu/h	r)		Ν	PM Traffic	Flow (pcu/hr)				Ν	S = 1940 ·	+ 100 (W-3	25) S =	2080 + 10	0 (W-3.25)	Note:		
		415					338 I			SM = S / (1 + 1.5 f/r)	SM =	(S - 230) / ((1 + 1.5 f/r)			
		L,	I				L,		I		AM	Peak	PM	Peak			
	1629	-				1013		-			1+2		1+2				
		1949						1922		Sum y	0.573		0.547				
		0						0		L (s)	8		8				
										C (s)	82		82				
										practical y	0.812		0.812				
										R.C. (%)	42%		48%				
1		2				3				4				5			
	+	. D _(P) ►		C2 C1													
A1 A2	•			ĹĻ													
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AM G :	=	I/G = 5	G =		I/G =	5	G =		I/G =		G =		I/G =		G =		
G :	-	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G :	=	I/G = 5	G =		I/G =	5	G =		I/G =		G =		I/G =		G =		
G	=	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

Junction:	Ng Lau Road	/ Lam Tei	Intercha	inae											Job Nu	mber:	J7265
Scenario:	With Approve	ed Scheme		<u> </u>												P.	11
Design Year:	2033	Designe	ed By:				-	Checke	d By:				-	Date:	21	Feb 20	24
										AM Peak					PM Peak		-
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Lam Tei Intercha	nge EB	SA	A2	1	4.00				2155	815	0.378			2155	507	0.235	
		SA	A3	1	4.00				2155	814	0.378			2155	506	0.235	
Lam Tei Intercha	nge WB	SA+LT	B1	1	4.00	15.0		0	2015	950	0.471	0.471	0	2015	937	0.465	0.465
		SA	B2	1	4.00				2155	1017	0.472			2155	1002	0.465	
Na Lau Dood CD		1.7	_														
Ng Lau Road SB		1.1	C1	2	5.50	10.0		100	1883	209	0.111	0.111	100	1883	169	0.090	0.090
		LI	C2	2	5.00	15.0		100	2050	228	0.111		100	2050	183	0.089	
pedestrian pha	ISE		D _(P)	1		min c	rossing	time =	5	sec	GM +	13	sec F	GM =	18	sec	
			E _(P)	2		min c	rossing	time =	10	sec	GM +	12	sec F	GM =	22	sec	
-																	
AM Traffic Flow (pcu/h	r)		N	PM Traffic I	low (pcu/hr)				Ν	S = 1940 ·	+ 100 (W-3.	.25) S =	2080 + 10	0 (W-3.25)	Note:		
	437						352			SM = S / (1 + 1.5 f/r)	SM =	(S - 230) / (1 + 1.5 f/r)			
	L		ļ				L		I		AM	Peak	PM	Peak			
→	1629					1013					1+2		1+2				
		1967						1939		Sum y	0.582		0.555				
		0						0		L (s)	8		8				
										C (s)	82		82				
										practical y	0.812		0.812				
										R.C. (%)	39%		46%				
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AM G =	= 1/C	G= 5	G =		I/G =	5	G =		I/G =		G =		I/G =		G =		
G =	= 1/0	3 =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G =	= 1/0	G= 5	G =		I/G =	5	G =		I/G =		G =		I/G =		G =		
G =	= I/C	G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

Junction:	Ng Lau Ro	ad / Lam Tei	Intercha	ange										-	Job Nu	mber:	J7265
Scenario:	With Propo	sed Scheme														P.	12
Design Year:	2033	Designe	ed By:				-	Checke	d By:				-	Date:	2	1 Feb 20	24
								r		AM Peak			<u> </u>		PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Lam Tei Intercha	inge EB	SA	A2	1	4.00				2155	815	0.378			2155	507	0.235	
		SA	A3	1	4.00				2155	814	0.378			2155	506	0.235	
			Ĺ														
			Ĺ														
Lam Tei Intercha	inge WB	SA+LT	B1	1	4.00	15.0	<u> </u>	0	2015	966	0.479	0.479	0	2015	949	0.471	0.471
		SA	B2	1	4.00				2155	1034	0.480	<u> </u>	<u> </u>	2155	1015	0.471	
					<u> </u>							<u> </u>	<u> </u>		<u> </u>		
Ng Lau Road SB		LT	C1	2	5.50	10.0	<u> </u>	100	1883	228	0.121	0.121	100	1883	177	0.094	0.094
		LT	C2	2	5.00	15.0	<u> </u>	100	2050	249	0.121	<u> </u>	100	2050	193	0.094	
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pedestrian pha	ise		D(P)	1	──	min c	rossing	time =	5	sec (GM +	13	sec F	GM =	18	sec	
			E _(P)	2	—	min c	rossing	time =	10	sec	GM +	12	sec F	GM =	22	sec	
			──		──	──			 '	──		 '	──		──	──	
			├───	──	──	──			 '	├───		 '	──		──	──	
			┝───	──	──	──			 '	├───		 '	──		—	──	
			┝───	──	──	──			 '	├───		 '	┣───		—	──	
			┝───	──	──	──			 '	├───		 '	──		—	──	
		<u>_</u>	L		L	L			<u> </u>	<u> </u>			<u> </u>		<u> </u>	<u> </u>	<u> </u>
AM Traffic Flow (pcu/hr	r)		N	PM Traffic F	Flow (pcu/hr)				N	S = 1940 ·	+ 100 (W-3	.25) S =	= 2080 + 10	00 (W-3.25)	Note:		
	477						370 			SM = S / (1 + 1.5 f/r)	SM =	(S - 230) / ((1 + 1.5 f/r)	I.		
	L	→	I				L		1	ſ	AM	Peak	PM	Peak			
$ \longrightarrow $	1629					1013		-	!		1+2		1+2				
		2000						1964		Sum y	0.601	 '	0.565	<u> </u>	1		
		0						0	I	L (s)	8	 '	8				
									ŀ	C (s)	82	 '	82	<u> </u>	1		
									I	practical y	0.812	 '	0.812	<u> </u>	1		
										R.C. (%)	35%	<u> </u>	44%		L		
1		2				3				4				5			
	▲ D _(P)			C2 C1					I				I				
A1	•			ĹĻ					I				I				
	♣ B2 ♣ B1 B1								I				I				
	Ļ			E(P) ♥					I				I				
													I				
AM G =	-	I/G = 5	G =		I/G =	5	G =		I/G =		G =		I/G =		G =		
G =		I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G =	-	VG = 5	G =		I/G =	5	G =		I/G =		G =		I/G =		G =		
G =	-	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

Signal Junction Analysis

Location	Tsing Lun	Road / Hor	ng Po Road	l / Lam Te	i Interchang	е						
Scenario	Existing C	Condition									Page	13
Design Ye	ear	2023	-		Job Numb	er	J7265			Date	21 Februa	ary 2024
AM Peak											-	
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c *		
From A	101	109	760	197					1167	253		
From B	181	0	76	21					278	1231		
From C	504	41	10	29					584	610		
From D	328	39	53	1					421	946		
From E												
From F												
From G												
From H												
Total	1114	189	899	248					2450			
					* q _c in (existing cond	dition is adjusted	d due to Tem	porary Traffic	Arrangement	-	
PM Peak												
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c *		
From A	204	90	692	311					1297	201		
From B	91	0	67	22					180	1350		
From C	404	32	18	37					491	720		
From D	160	26	33	2					221	839		
From E												
From F												
From G												
From H												
Total	859	148	810	372					2189	1	1	
							* q _c	in existing c	ondition is adj	usted for TTA	-	

Legend	
Arm	Road (in clockwise order)
Α	Slip Road from Lam Tei Interchange
В	Access Road from Siu Hong Station
С	Tsing Lun Road
D	Hong Po Road
Е	
F	
G	
н	

Geometric Parameters												
Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø (°)	S					
From A	10.0	7.3	20.0	10.0	55	45	0.4					
From B	9.0	6.8	28.0	4.0	55	19	0.9					
From C	11.5	7.8	100.0	9.0	55	23	0.7					
From D*	6.0	4.5	27.0	6.0	55	10	0.4					
From E												
From F												
From G												
From H												

Predictive Equation $Q_E = K(F - f_cq_c)$

- Q_E
 Entry Capacity

 q_c
 Circulating Flow across the Entry
- $\begin{array}{lll} \mathsf{K} & = 1\mbox{-}0.00347(\ensuremath{\varnothing}\mbox{-}30)\mbox{-}0.978[(1/r)\mbox{-}0.05] \\ \mathsf{F} & = 303x_2 \end{array}$
- $f_c = 0.210t_D(1{+}0.2x_2)$
- $t_D = 1+0.5/(1+M)$
- M = exp[(D-60)/10]
- $x_2 = v+(e-v)/(1+2S)$
- S = 1.6(e-v)/L

e Entry Width v Approach Half Width

* Parameter in existing condition is adjusted for TTA

Limitation

v	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
S	Sharpness of Flare	0.0 - 3.0

4.0 - 15.0 m

							Q _E		Entry	Flow	RI	=C
Arm	x ₂	М	t _D	К	F	f _c	AM	PM	AM	PM	AM	PM
From A	8.748	0.607	1.311	0.948	2651	0.757	2331	2369	1167	1297	0.501	0.548
From B	7.597	0.607	1.311	1.051	2302	0.694	1522	1435	278	180	0.183	0.125
From C	9.398	0.607	1.311	1.063	2848	0.793	2514	2421	584	491	0.232	0.203
From D	5.333	0.607	1.311	1.082	1616	0.569	1166	1232	421	221	0.361	0.179
From E												
From F												
From G												
From H												

Location	Tsing Lun Road	I / Hong Po Ro	oad / Lam Tei	Interchange
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Scenario Wit	hout Development				Page	14
Design Year	2033	Job Number	J7265	Date	21 February	/ 2024

AM Peak										
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c
From A	329	0	995	503					1827	427
From B	196	0	86	20					302	2148
From C	796	50	11	121					978	1048
From D	699	56	310	0					1065	1382
From E										
From F										
From G										
From H										
Total	2020	106	1402	644					4172	

PM Peak										
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c
From A	418	0	801	604					1823	267
From B	107	0	76	25					208	2009
From C	539	41	20	146					746	1154
From D	431	40	166	0					637	1125
From E										
From F										
From G										
From H										
Total	1495	81	1063	775					3414	

L	ρ	a	ρ	n	d

Legenu		
Arm	Road (in clockwise order)	
А	Slip Road to Lam Tei Interchange	
В	Access Road to Siu Hong Station	
С	Tsing Lun Road	
D	Hong Po Road	
Е		
F		
G		
Н		

Geometric Parameters												
Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø (°)	S					
From A	11.0	7.8	30.0	10.0	55	20	0.5					
From B	9.0	6.8	28.0	6.0	55	19	0.6					
From C	11.5	7.8	100.0	9.0	55	23	0.7					
From D	14.0	8.5	40.0	10.0	55	10	0.9					
From E												
From F												
From G												
From H												

Predictive Equation $Q_E = K(F - f_cq_c)$

Q_E	Entry Capacity
$\mathbf{q}_{\mathbf{c}}$	Circulating Flow across the Entry
К	= 1-0.00347(Ø-30)-0.978[(1/r)-0.05]

- Κ $= 303x_2$ F
- $= 0.210t_D(1+0.2x_2)$ \mathbf{f}_{c}
- = 1+0.5/(1+M) \mathbf{t}_{D}
- Μ $= \exp[(D-60)/10]$
- = v+(e-v)/(1+2S) \mathbf{X}_2
- S = 1.6(e-v)/L

е	Entry Width	4.0 - 15.0 m
v	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
S	Sharpness of Flare	0.0 - 3.0

							Q _E		Entry Flow		RFC	
Arm	x ₂	М	t _D	К	F	f _c	AM	PM	AM	PM	AM	PM
From A	9.381	0.607	1.311	1.051	2842	0.792	2632	2765	1827	1823	0.694	0.659
From B	7.812	0.607	1.311	1.051	2367	0.706	895	998	302	208	0.337	0.208
From C	9.398	0.607	1.311	1.063	2848	0.793	2144	2055	978	746	0.456	0.363
From D	10.493	0.607	1.311	1.094	3179	0.853	2188	2428	1065	637	0.487	0.262
From E												
From F												
From G												
From H												

Location	Tsing Lun	Road / Hor	ng Po Road	d / Lam Tei	Interchang	е						
Scenario	With Appr	oved Schei	me								Page	15
Design Ye	ear	2033	-		Job Numb	er	J7265	-		Date	21 Februa	ary 2024
AM Peak										-		
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c		
From A	347	0	995	503					1845	435		
From B	196	0	86	20					302	2174		
From C	798	50	11	121					980	1066		
From D	699	56	318	0					1073	1402		
From E												
From F												
From G												
From H												
Total	2040	106	1410	644					4200			

PM Peak										
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c
From A	435	0	801	604					1840	269
From B	107	0	76	25					208	2028
From C	541	41	20	146					748	1171
From D	0	40	168	0					208	1144
From E										
From F										
From G										
From H										
Total	1083	81	1065	775					3004	

Legend

Arm	Road (in clockwise order)
А	Slip Road to Lam Tei Interchange
В	Access Road to Siu Hong Station
С	Tsing Lun Road
D	Hong Po Road
Е	
F	
G	
н	

Geometric Parameters Arm e (m) v (m) r (m) L (m) D (m) Ø (°) S From A 11.0 7.8 30.0 10.0 55 20 0.5 From B 9.0 6.8 28.0 4.0 55 19 0.9 From C 11.5 7.8 100.0 9.0 55 23 0.7 From D 14.0 8.5 40.0 10.0 55 10 0.9 From E From F From G From H

Predictive Equation $Q_E = K(F - f_cq_c)$

Q_E	Entry Capacity
q _c	Circulating Flow across the Entry

- = 1-0.00347(Ø-30)-0.978[(1/r)-0.05] Κ = 303x₂ F
- $= 0.210t_D(1+0.2x_2)$ \mathbf{f}_{c}
- \mathbf{t}_{D} = 1+0.5/(1+M)
- Μ $= \exp[(D-60)/10]$
- \mathbf{x}_2 = v+(e-v)/(1+2S)
- S = 1.6(e-v)/L

Limitation

е	Entry Width	4.0 - 15.0 m
v	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
S	Sharpness of Flare	0.0 - 3.0

							Q _E		Entry Flow		RFC	
Arm	x ₂	М	t _D	К	F	f _c	AM	PM	AM	PM	AM	PM
From A	9.381	0.607	1.311	1.051	2842	0.792	2625	2764	1845	1840	0.703	0.666
From B	7.597	0.607	1.311	1.051	2302	0.694	834	941	302	208	0.362	0.221
From C	9.398	0.607	1.311	1.063	2848	0.793	2129	2041	980	748	0.460	0.367
From D	10.493	0.607	1.311	1.094	3179	0.853	2169	2410	1073	208	0.495	0.086
From E												
From F												
From G												
From H												

Location	Tsing Lun Road / Hong Po Road / Lam Tei Interchange	
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Scenario With Pr	Page	16						
Design Year	2033		Job Numb	ber <u>J7265</u>	_	Date	21 Februar	ry 2024
AM Peak							_	
	T D	T O	T D T C	T F T O	T 11 T 11			

Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c
From A	380	0	995	503					1878	450
From B	196	0	86	20					302	2222
From C	801	50	11	121					983	1099
From D	699	56	333	0					1088	1438
From E										
From F										
From G										
From H										
Total	2076	106	1425	644					4251	

PM Peak										
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c
From A	460	0	801	604					1865	272
From B	107	0	76	25					208	2056
From C	545	41	20	146					752	1196
From D	0	40	171	0					211	1173
From E										
From F										
From G										
From H										
Total	1112	81	1068	775					3036	

Legend

Logona	
Arm	Road (in clockwise order)
А	Slip Road to Lam Tei Interchange
В	Access Road to Siu Hong Station
С	Tsing Lun Road
D	Hong Po Road
Е	
F	
G	
н	

Geometric Parameters Arm e (m) v (m) r (m) L (m) D (m) Ø (°) S From A 11.0 7.8 30.0 10.0 55 20 0.5 From B 9.0 6.8 28.0 4.0 55 19 0.9 From C 11.5 7.8 100.0 9.0 55 23 0.7 From D 14.0 8.5 40.0 10.0 55 10 0.9 From E From F From G From H

Predictive Equation $Q_E = K(F - f_cq_c)$

Q _E	Entry Capacity
q _c	Circulating Flow across the Entry

- $\begin{array}{ll} \mathsf{K} & = 1\mbox{-}0.00347(\ensuremath{\varnothing}\mbox{-}30)\mbox{-}0.978[(1/r)\mbox{-}0.05] \\ \mathsf{F} & = 303x_2 \end{array}$
- $f_c = 0.210t_D(1+0.2x_2)$
- $t_D = 1+0.5/(1+M)$
- $M = \exp[(D-60)/10]$
- $x_2 = v+(e-v)/(1+2S)$
- S = 1.6(e-v)/L

imitatio	on	
е	Entry Width	4.0 - 15.0 m
v	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
S	Sharpness of Flare	0.0 - 3.0

Ratio-of-Flow to Capacity (RFC)

							Q _E		Entry Flow		RFC	
Arm	x ₂	М	t _D	К	F	f _c	AM	PM	AM	PM	AM	PM
From A	9.381	0.607	1.311	1.051	2842	0.792	2613	2761	1878	1865	0.719	0.675
From B	7.597	0.607	1.311	1.051	2302	0.694	799	920	302	208	0.378	0.226
From C	9.398	0.607	1.311	1.063	2848	0.793	2101	2020	983	752	0.468	0.372
From D	10.493	0.607	1.311	1.094	3179	0.853	2136	2383	1088	211	0.509	0.089
From E												
From F												
From G												
From H												

L

Location	Lam Tei I	nterchange										
Scenario	Existing (Condition									Page	17
Design Ye	ear	2023	_		Job Numb	er	J7265			Date	21 Februa	ary 2024
AM Peak											_	
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c		
From A	16	0	878						894	605		
From B	292	3	275						570	907		
From C	317	589	13						919	311		
From D												
From E												
From F												
From G												

PM Peak										
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c
From A	7	0	891						898	448
From B	266	0	380						646	924
From C	175	422	26						623	273
From D										
From E										
From F										
From G										
From H										
Total	448	422	1297						2167	

F

Legend

Total

Legena	
Arm	Road (in clockwise order)
А	Slip Road to Castle Peak Road
В	Slip Road to Tuen Mun Road
С	Slip Road to Tsing Lun Road
D	
Е	
F	
G	
Н	

625

592

1166

Geometric Parameters

Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø (°)	S
rom A	7.3	7.3	40.0	1.0	45	22	0.0
rom B	8.8	7.3	65.0	3.0	45	26	0.8
rom C	7.7	6.0	100.0	8.0	45	17	0.3
rom D							
rom E							
rom F							
rom G							
rom H							

2383

Predictive Equation $Q_E = K(F - f_cq_c)$

Q_E	Entry Capacity
q _c	Circulating Flow across the Entry
к	= 1-0.00347(Ø-30)-0.978[(1/r)-0.05]
F	= 303x ₂
f _c	$= 0.210t_{D}(1+0.2x_{2})$
t _D	= 1+0.5/(1+M)
М	= exp[(D-60)/10]
x ₂	= v+(e-v)/(1+2S)
S	= 1.6(e-v)/L

Limitation Entry Width 4.0 - 15.0 m е 2.0 - 7.3 m Approach Half Width ۷ Entry Radius 6.0 - 100.0 m r Effective Length of Flare 1.0 - 100.0 m L D Inscribed Circle Diameter 15 - 100 m 10° - 60° Ø Entry Angle s Sharpness of Flare 0.0 - 3.0

							C	ζE	Entry	Flow	RI	=C
Arm	X ₂	М	t _D	К	F	f _c	AM	PM	AM	PM	AM	PM
From A	7.300	0.223	1.409	1.052	2212	0.728	1864	1984	894	898	0.480	0.453
From B	7.877	0.223	1.409	1.048	2387	0.762	1777	1763	570	646	0.321	0.366
From C	7.012	0.223	1.409	1.084	2125	0.711	2064	2093	919	623	0.445	0.298
From D												
From E												
From F												
From G												
From H												

Location	Lam Tei I	nterchange										
Scenario	Without D	Developmen	ıt								Page	18
Design Ye	ear	2033	-		Job Numb	ber	J7265	_		Date	21 Febru	ary 2024
Am	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c	1	
From A	18	0	1162						1180	995		
From B	323	0	775						1098	1192		
From C	632	983	12						1627	341		
From D												
From E												
From F												
From G												
From H												
Total	973	983	1949						3905			

PM Peak										
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c
From A	15	0	1137						1152	703
From B	283	0	759						1042	1178
From C	379	677	26						1082	298
From D										
From E										
From F										
From G										
From H										
Total	677	677	1922						3276	

F

Leaend

Logona	
Arm	Road (in clockwise order)
А	Slip Road to Castle Peak Road
В	Slip Road to Tuen Mun Road
С	Slip Road to Tsing Lun Road
D	
Е	
F	
G	
Н	

Geometric Parameters

Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø (°)	S
rom A	7.3	7.3	40.0	1.0	45	22	0.0
rom B	8.8	7.3	65.0	3.0	45	26	0.8
rom C	7.7	6.0	100.0	8.0	45	17	0.3
rom D							
rom E							
rom F							
rom G							
rom H							

Predictive Equation $Q_E = K(F - f_cq_c)$

Q_E	Entry Capacity
q _c	Circulating Flow across the Entry
K F	= 1-0.00347(∅-30)-0.978[(1/r)-0.05] = 303x ₂
f _c	$= 0.210t_D(1+0.2x_2)$
t _D	= 1+0.5/(1+M)
М	= exp[(D-60)/10]
x ₂	= v+(e-v)/(1+2S)
S	= 1.6(e-v)/L

Limitation Entry Width е

е	Entry Width	4.0 - 15.0 m
v	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
s	Sharpness of Flare	0.0 - 3.0

							Q _E		Entry Flow		RFC	
Arm	x ₂	М	t _D	К	F	f _c	AM	PM	AM	PM	AM	PM
From A	7.300	0.223	1.409	1.052	2212	0.728	1565	1789	1180	1152	0.754	0.644
From B	7.877	0.223	1.409	1.048	2387	0.762	1549	1560	1098	1042	0.709	0.668
From C	7.012	0.223	1.409	1.084	2125	0.711	2041	2074	1627	1082	0.797	0.522
From D												
From E												
From F												
From G												
From H												

Location	Lam Tei Ir	nterchange										
Scenario	With Appr	oved Schei	me								Page	19
Design Ye	ar	2033	-		Job Numb	er	J7265	-		Date	21 Februa	ary 2024
AM Peak												
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c]	
From A	18	0	1168						1186	1008		
From B	323	0	787						1110	1198		
From C	637	996	12						1645	341		
From D												
From E												
From F												
From G												
From H												
Total	978	996	1967						3941			

PM Peak										
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c
From A	15	0	1142						1157	711
From B	283	0	771						1054	1183
From C	382	685	26						1093.462	298
From D										
From E										
From F										
From G										
From H										
Total	680	685	1939						3304.462	

F F

Leaend

Logona	
Arm	Road (in clockwise order)
А	Slip Road to Castle Peak Road
В	Slip Road to Tuen Mun Road
С	Slip Road to Tsing Lun Road
D	
Е	
F	
G	
Н	

Geometric Parameters

Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø(°)	S
rom A	7.3	7.3	40.0	1.0	45	22	0.0
rom B	8.8	7.3	65.0	3.0	45	26	0.8
rom C	7.7	6.0	100.0	8.0	45	17	0.3
rom D							
rom E							
rom F							
rom G							
rom H							

Predictive Equation $Q_E = K(F - f_cq_c)$

Q _E	Entry Capacity
q _c	Circulating Flow across the Entry
к	= 1-0.00347(Ø-30)-0.978[(1/r)-0.05]
F	$= 303X_2$
f _c	$= 0.210t_{D}(1+0.2x_{2})$
t _D	= 1+0.5/(1+M)
М	= exp[(D-60)/10]
x ₂	= v+(e-v)/(1+2S)
S	= 1.6(e-v)/L

Limitation

milain			
е	Entry Width	4.0 - 15.0 m	
v	Approach Half Width	2.0 - 7.3 m	
r	Entry Radius	6.0 - 100.0 m	
L	Effective Length of Flare	1.0 - 100.0 m	
D	Inscribed Circle Diameter	15 - 100 m	
Ø	Entry Angle	10° - 60°	
S	Sharpness of Flare	0.0 - 3.0	

Ratio-of-Flow to Capacity (RFC)

							Q _E		Entry Flow		RFC	
Arm	X ₂	М	t _D	К	F	f _c	AM	PM	AM	PM	AM	PM
From A	7.300	0.223	1.409	1.052	2212	0.728	1555	1783	1186	1157	0.762	0.649
From B	7.877	0.223	1.409	1.048	2387	0.762	1544	1556	1110	1054	0.719	0.677
From C	7.012	0.223	1.409	1.084	2125	0.711	2041	2074	1645	1093	0.806	0.527
From D												
From E												
From F												
From G												
From H												
Roundabout Analysis

Location	Lam Tei Int	erchange										
Scenario	With Propo	sed Scher	me								Page	20
Design Ye	ear	2033	-		Job Numb	er	J7265	-		Date	21 Febru	ary 2024
AM Peak												
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c]	
From A	18	0	1180						1198	1031	1	
From B	323	0	808						1131	1210		
From C	647	1019	12						1678	341		
From D												
From E												
From F												
From G												
From H												
Total	988.4237	1019	2000						4007			

PM Peak										
Arm	To A	To B	To C	To D	To E	To F	To G	To H	Total	q _c
From A	15	0	1150						1165	721
From B	283	0	788						1071	1191
From C	387	695	26						1108	298
From D										
From E										
From F										
From G										
From H										
Total	685	695	1964						3344	

F

Leaend

Logona	
Arm	Road (in clockwise order)
А	Slip Road to Castle Peak Road
В	Slip Road to Tuen Mun Road
С	Slip Road to Tsing Lun Road
D	
Е	
F	
G	
Н	

Geometric Parameters

Arm	e (m)	v (m)	r (m)	L (m)	D (m)	Ø (°)	S
rom A	7.3	7.3	40.0	1.0	45	22	0.0
rom B	8.8	7.3	65.0	3.0	45	26	0.8
rom C	7.7	6.0	100.0	8.0	45	17	0.3
rom D							
rom E							
rom F							
rom G							
rom H							

Predictive Equation $Q_E = K(F - f_cq_c)$

Q_E	Entry Capacity
q _c	Circulating Flow across the Entry
к	= 1-0.00347(Ø-30)-0.978[(1/r)-0.05]
F	$= 303x_2$
f _c	$= 0.210t_D(1+0.2x_2)$
t _D	= 1+0.5/(1+M)
М	= exp[(D-60)/10]
x ₂	= v+(e-v)/(1+2S)
S	= 1.6(e-v)/L

Limitation

е	Entry Width	4.0 - 15.0 m
v	Approach Half Width	2.0 - 7.3 m
r	Entry Radius	6.0 - 100.0 m
L	Effective Length of Flare	1.0 - 100.0 m
D	Inscribed Circle Diameter	15 - 100 m
Ø	Entry Angle	10° - 60°
S	Sharpness of Flare	0.0 - 3.0

Ratio-of-Flow to Capacity (RFC)

							Q _E		Entry	Flow	RFC		
Arm	x ₂	М	t _D	К	F	f _c	AM	PM	AM	PM	AM	PM	
From A	7.300	0.223	1.409	1.052	2212	0.728	1538	1775	1198	1165	0.779	0.656	
From B	7.877	0.223	1.409	1.048	2387	0.762	1535	1550	1131	1071	0.737	0.691	
From C	7.012	0.223	1.409	1.084	2125	0.711	2041	2074	1678	1108	0.822	0.534	
From D													
From E													
From F													
From G													
From H													

						-											
Junction:	Lam Tei Inte	rchange / C	Castle Pe	eak Roa	ıd – Lam	ı Tei									Job Nu	mber:	J7265
Scenario:	Existing Con	dition														Ρ.	21
Design Year:	2023	Designe	ed By:					Checke	d By:					Date:	21	Feb 20	24
					-												
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
Castle Peak Roa	id -Lam Tei SB				0.05	00.0	Gradient	400	(pcu/hr)	(pcu/hr)	0.404	0.404	400	(pcu/hr)	(pcu/hr)	0.400	0.400
Castle I eak itoa		LT	A1	1,4	3.25	80.0		100	1904	363	0.191	0.191	100	1904	248	0.130	0.130
		SA	A2	1,4	3.25				2080	156	0.075			2080	132	0.063	
		SA+RT	A3	1	3.25	22.5		64	1995	150	0.075		56	2005	127	0.063	
		RT	A4	1	3.25	20.0		100	1935	145	0.075		100	1935	122	0.063	
Slip Road from L	am Tei Interchar	ige LT	B1	2,3	3.50	8.0		100	1655	162	0.098		100	1655	115	0.069	
		LT+RT	B2	2,3	3.50	30.0		100	2005	255	0.127		100	2005	191	0.095	
		RT	B3	2,3	3.50	26.0		100	1990	253	0.127	0.127	100	1990	189	0.095	0.095
		RT	B4	2,3	3.50	23.0		100	1976	229	0.116		100	1976	124	0.063	
Castle Peak Roa	Id NB	LT	C1	3,4	3.50	14.0		100	1775	454	0.256		100	1775	375	0.211	
		LT	C2	3,4	3.50	19.0		100	1951	499	0.256		100	1951	413	0.212	
		SA	D1	4	3,50				2105	137	0.065			2105	223	0.106	
		SA	D2	4	3.50				2105	136	0.065			2105	222	0 105	
		0/1	DL		0.00				2100	100	0.000			2100	LLL	0.100	
pedestrian pha	ase		E _(P)	1,3		min c	rossing	time =	6	sec	GM +	12	sec F	GM =	18	sec	
			F _(P)	1		min c	rossing	time =	5	sec	GM +	7	sec F	GM =	12	sec	
			G _(P)	2		min c	rossing	time =	6	sec	GM +	11	sec F	GM =	17	sec	
			H _(P)	2		min c	rossing	time =	5	sec	GM +	9	sec F	GM =	14	sec	
AM Traffic Flow (pcu/h	r)			PM Traffic	Elow (pcu/hr)										Note:		
	,	<pre></pre>	N						N	S = 1940 +	- 100 (W-3.:	25) S =	= 2080 + 10	0 (W-3.25)			
										SM = S / (1 + 1.5 f/r)	SM =	(S - 230) /	(1 + 1.5 f/r)			
³⁶	3		_		²⁴⁸			-	_		AM	Peak	PM	Peak			
	210	273			\rightarrow	188		445			1,4+2,3		1,4+2,3				
24	1	953			193			788		Sum y	0.318		0.225				
	← →	_				-				L (s)	21		21				
	162 508	229				115	380	124		C (s)	110		110				
							·			practical y	0.728		0.728				
										R.C. (%)	129%		223%				
														_			
1	t	2				3				4	t			5			
A1 A2	* •	G _(P)	÷							A1 A2							
A3													D2 D1				
		• ¬•	┯╸┍╸┍	•		• ¬•	┝┯┿┍┿┍╵	• ==	C2 C1			F	C2 C1				
F _(P) ↓ ←	*				←>			•			←;	•					
E	P)	B1	B2 B3 B4		H _(P)	B1	B2 B3 B4				E _(P)						
AM G:	= 1/	G= 15	G =		I/G =		G =		I/G =	8	G =		I/G =		G =		
G	= 1/	G= 15	G =		I/G =		G =		I/G =	7	G =		I/G =	7	G =		
PM G	= 14	G= 15	G =		I/G =		G =		I/G =	VG = 8 G =				//G = G =			
	M G= 15							.0 -	G= 7 G=			/G = 7 c					
G	- 1/	0 = IJ	G =		i/G =		G =		vG =	'	G =		1/G =	'	G =		

					9		,									
Junction:	Lam Tei Interchange / C	Castle Pe	eak Roa	d – Lam	Tei									Job Nu	mber:	J7265
Scenario:	Without Development														Ρ.	22
Design Year:	2033 Designe	ed By:					Checke	d By:					Date:	21	Feb 20	24
									AM Book					BM Book		
	Approach	Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	Flow	y value	Critical y	Turning %	Sat. Flow	Flow	y value	Critical y
Castle Peak Roa	d SB LT	A1	1.4	3.50	80.0	Gradient	100	1929	444	0.230		100	1929	273	0.142	
	SA+RT	A2	1	3 50	30.0		0	2105	288	0 137	0 137	0	2105	231	0 110	0 1 1 0
	RT	A3	1	3.50	25.0		100	1986	166	0.084		100	1986	118	0.059	
	RT	A4	1	3.50	20.0		100	1958	163	0.083		100	1958	116	0.059	
Slip Road from	LT+(To YL Hwy)RT	B2	2,3	3.50	30.0		100	2105	553	0.263		100	2005	391	0.195	0.195
Lam Tei Intercha	nge (To YL Hwy) RT	B3	2,3	3.50	25.0		100	1986	522	0.263		100	2086	406	0.195	
	(To CPR) RT	B4	2,3	3.50	20.0		100	1958	315	0.161		100	1958	148	0.076	
Castle Peak Roa	d NB LT	C1	3,4	3.50	14.0		100	1775	588	0.331	0.331	100	1775	485	0.273	
	LT	C2	3,4	3.50	19.0		100	1951	646	0.331		100	1951	533	0.273	
	SA	D1	4	3.50				2105	220	0.105			2105	330	0.157	0.157
	SA	D2	4	3.50				2105	220	0.105			2105	330	0.157	
pedestrian pha	ise	D _(P)	1		min c	rossing	time =	6	sec	GM +	12	sec F	GM =	18	sec	
		E _(P)	1,4		min c	rossing	time =	5	sec	GM +	7	sec F	GM =	12	sec	
		F _(P)	2		min c	rossing	time =	6	sec	GM +	11	sec F	GM =	17	sec	
		G _(P)	2,3		min c	rossing	time =	5	sec	GM +	9	sec F	GM =	14	sec	
AM Traffic Flow (pcu/h	r)	N	PM Traffic I	Flow (pcu/hr)				N	S - 1940 -	100 /W-3	25) S-	- 2080 ± 10	0 (W-3 25)	Note:		
	×						\		SM = S / (1	1 + 1 5 f/r)	SM =	(S - 230) / ((1 + 1.5 f/r)			
▲ 44	4			▲ 273					011 - 07 (ΔM	Peak	PM	Peak			
	288 440			→	231		660			1+3.4	oun	1+2.3+4	oun			
¥ 32	9 1234			↓ 234			1018		Sum v	0 468		0 462				
									L (s)	38		26				
	208 867 315				146	651	► 148		C (s)	110		110				
									practical v	0.589		0.687				
									R.C. (%)	26%		49%				
					<u>_</u>								-			
1 A1		≜			3	≜			4	t			5			
	G _(P)	ŧ			G _(P)	ŧ			A1		←	D2				
A4								C2			<u> </u>	D1 C2				
]	╈┎╸	•		•	╈┍╸┌	ţ	C1			ţ	C1				
^U (P) ↓ ← E	▶) B1	 B2 B3 B4		← ► F _(P)	B1	 B2 B3 B4				← → Err>	•					
-(F	,			• /	L					-(P)						
AM G =	= 1/G = 15	G =	7	I/G =	11	G =		I/G =	_	G =		I/G =	7	G =		
G = 1/G = 15 G =				I/G =		G =		I/G =	VG = 7 G =			I/G =	/	G =		
AM G =	M G = 1/G = 15			G = 1/G = G =				I/G =	VG = 1 G = V/G = 7				1	G =		
G =	= I/G = 15	G =	1	I/G =	11	G = 1/G = G = 1/G				I/G =	1	G =				

					0		,									
Junction:	Lam Tei Interchange / C	astle P	eak Roa	ıd – Lam	Tei									Job Nu	mber:	J7265
Scenario:	With Approved Scheme														Ρ.	23
Design Year:	2033 Designe	ed By:					Checke	d By:					Date:	21	Feb 20	24
			1	-	ı.											
	Approach	Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
Castla Book Boo						Gradient		(pcu/hr)	(pcu/hr)				(pcu/hr)	(pcu/hr)		
Castle I cak itoa		A1	1,4	3.50	80.0		100	1929	444	0.230		100	1929	273	0.142	
	DT	A2	1	4.00	85.0		0	2155	288	0.134	0.134	0	2155	231	0.107	0.107
	PT	A3	1	3.50	25.0		100	1986	166	0.084		100	1986	118	0.059	
		A4	1	3.50	20.0		100	1958	163	0.083		100	1958	116	0.059	
Clip Dood from																
Silp Road from	LT+(18 YL Hwy)RT	B2	2,3	3.50	30.0		100	2105	557	0.265		100	2005	394	0.197	0.197
Lam Tei Intercha	nge (TO YL Hwy) RT	B3	2,3	3.50	25.0		100	1986	525	0.264		100	2086	409	0.196	
	(TO CPR) RT	B4	2,3	3.50	20.0		100	1958	317	0.162		100	1958	148	0.076	
	110															
Castle Peak Roa	d NB LT	C1	3,4	3.50	14.0		100	1775	591	0.333	0.333	100	1775	487	0.274	
	LT	C2	3,4	3.50	19.0		100	1951	649	0.333		100	1951	536	0.275	
	SA	D1	4	3.50				2105	220	0.105			2105	330	0.157	0.157
	SA	D2	4	3.50				2105	220	0.105			2105	330	0.157	
pedestrian pha	ise	E _(P)	1		min c	rossing	time =	6	sec	GM +	12	sec F	GM =	18	sec	
		F _(P)	1,4		min c	rossing	time =	5	sec	GM +	7	sec F	GM =	12	sec	
		G _(P)	2		min c	rossing	time =	6	sec	GM +	11	sec F	GM =	17	sec	
		$H_{(P)}$	2,3		min c	rossing	time =	5	sec	GM +	9	sec F	GM =	14	sec	
AM Traffic Flow (pcu/hi	r)	N	PM Traffic	Flow (pcu/hr)				N	1					Note:		
		N						N	S = 1940 +	- 100 (W-3.:	25) S =	= 2080 + 10	0 (W-3.25)			
									SM = S / (l + 1.5 f/r)	SM =	(S - 230) / ((1 + 1.5 f/r)			
44	4 	_ `						_ `		AM	Peak	PM	Peak			
$ \rightarrow $	288 440			\neg	231		660			1+3,4		1 + 2,3 + 4				
329	9 1240			234			1023		Sum y	0.467		0.460				
	←┐┌→┌→				• r	→ ┌─	•		L (s)	38		26				
	208 874 317				146	657	148		C (s)	110		110				
									practical y	0.589		0.687				
									R.C. (%)	26%		49%				
1	2				3				4				5			
A1	H _(P)	1			H _(P)	1			A1	t						
A2 A3		•				•					<u> </u>	C4				
A4			•				.	C2				C2				
E _{IP}			-				ţ.	C1			↓	C1				
°′ ↓ ← − − F _{(F}	• I B1	B2 B3 B4		G _(P)	B1	B2 B3 B4				← → F _(P)	•					
			7		44				I				-			
AM G =	= I/G = 15	G =	1	I/G =	11	G =		I/G =	-	G =		I/G =	1	G =		
G =	= I/G = 15	G =		I/G =		G =		I/G =	7	G =		I/G =	7	G =		
AM G =	= 1/G = 15	G =	G = 1/G = G =					I/G =	VG = 1 G = 1/G = 1				<i>'</i>	G =		
G =	= I/G = 15	G =	7	I/G =	11	G =		I/G =		G =		I/G =	7	G =		

Junction:	Lam Tei Inte	erchange / C	Castle P	eak Roa	ad – Lan	n Tei									Job Nu	mber:	J7265
Scenario:	With Propos	sed Scheme	9													Ρ.	24
Design Year:	2033	Designe	ed By:					Checke	d By:					Date:	2′	Feb 20	24
										AM Pook					PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow	Flow (ncu/br)	y value	Critical y	Turning %	Sat. Flow	Flow (ncu/br)	y value	Critical y
Castle Peak Roa	ad SB	LT	A1	1.4	3.50	80.0	Ordalorit	100	1929	444	0.230		100	1929	273	0.142	
		SA+RT	A2	1	4.00	85.0		0	2155	288	0.134	0.134	0	2155	231	0.107	0.107
		RT	A3	1	3.50	25.0		100	1986	167	0.084		100	1986	119	0.060	
		RT	A4	1	3.50	20.0		100	1958	164	0.084		100	1958	117	0.060	
Slip Road from	LT+(Te	o YL Hwy)RT	B2	2,3	3.50	30.0		100	2105	564	0.268		100	2005	397	0.198	0.198
Lam Tei Intercha	ange (To	YL Hwy) RT	B3	2,3	3.50	25.0		100	1986	532	0.268		100	2086	413	0.198	
		(To CPR) RT	B4	2,3	3.50	20.0		100	1958	320	0.163		100	1958	149	0.076	
Castle Peak Roa	ad NB	LT	C1	3,4	3.50	14.0		100	1775	595	0.335	0.335	100	1775	490	0.276	
		LT	C2	3,4	3.50	19.0		100	1951	655	0.336		100	1951	539	0.276	
		SA	D1	4	3.50				2105	220	0.105			2105	330	0.157	0.157
		SA	D2	4	3.50				2105	220	0.105			2105	330	0.157	
nedestrian pha	926		Em	1		min c	rossina	time –	6	SAC	GM +	12	sec F	GM -	18	SAC	
pedestriari pric	450		С(P) F(P)	1 /		min c	rossing	time -	5	500		7	sec FGM =		10	500	
			G _(P)	2		min c	rossing	time =	6	Sec	GM +	11	sec F	GM =	17	Sec	
			H _(P)	23		min c	rossing	time =	5	sec	GM +	9	sec F	GM =	14	sec	
				2,0			oconig		U	000		Ū	0001	0		000	
AM Traffic Flow (pcu/h	nr)		N	PM Traffic	Flow (pcu/hr))			N						Note:		
										S = 1940 +	+ 100 (W-3.	25) S =	2080 + 10	0 (W-3.25)			
4 4	4				▲ 273					SIVI = 37 (1 + 1.5 1/1)	Sivi =	(3 - 230) / ([1 + 1.5 I/I)			
	288	440			-	231		660	_		1+3.4	reak	1+23+4	reak			
¥ 33	1	1250			¥ 236			1029		Sumv	0.469		0 462				
											38		26				
	209 887	320				146	664	► 149		C (s)	110		110				
		1					ļ			practical y	0.589		0.687				
										R.C. (%)	26%		49%				
1		2				3				4				5			
A1	t	ے Hm	ţ			J Hon	<u>†</u>			- A1	t			5			
A2 A3	-	(*)	*			(P)	*					<u> </u>	C4				
A4			┝┯╪┍╪┍┥	•		•	╘╌┾┌┾┌╴		C2			<u> </u>	C3 C2				
E _(P)																	
	• •	B1	B2 B3 B4		G _(P)	B1	B2 B3 B4				← • F _(P)	•					
AM G-		I/G = 15	G =	7	I/G =	11	G =		I/G =	L	G =		I/G =	7	G =		
G =	. 1	/G = 15	G =		I/G =		G =		I/G =	7	G =		I/G =	7	G =		
AM G =	- 1	I/G = 15	G =		I/G =		G =		I/G =	7	G =		I/G =	7 G =			
G =	G = 1/G = 15			7	I/G =	11	G =	G = 1/G = G =				I/G = 7 G =					

Junction:	Tsing Lur	<u>1 Road/ Tsz Tir</u>	ו Road												Job Nu	mber:	J7265
Scenario:	Existing	<u>Condition</u>						Chaoka						Doto:		P.	25
Design rear.	2023	Designe	на ву.				•	Спеске	а Бу.					Date.		Fed 20	24
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
Tsing Lun Roa		I T+SA		\vdash		10.0	Gradient		(pcu/hr)	(pcu/hr)		0.047	10	(pcu/hr)	(pcu/hr)	. 170	
	0.00	RT	A1 42		3.65	13.0	┟───┦	4	19/1	480 241	0.247	0.247	12	1953	337	0.173	0 187
			<u> 74</u>		3.05	10.0	├ ──┦	100	1921	341	0.177		100	2120	391	0.107	0.107
Tsing Lun Roa	ıd NB	LT+SA	C1	2	3.30	23.0		37	1899	115	0.061	0.061	33	1904	120	0.063	0.063
		SA	C2	2	3.40	[['	['	2095	126	0.060			2095	132	0.063	
		RT	C3	2	3.40	15.0	<u> '</u>	100	1905	7	0.004		100	1905	17	0.009	
Access Road (IT SALDT	 	\vdash	+	<u> </u>	──'	'		<u> </u>							
Siu Hong Cour	:o rt WB	LITOATI	D1	3	3.50	15.0	──'	90	1803	67	0.037	0.037	88	1806	58	0.032	0.032
Old Hong Coa.				<u> </u>	+		┝──┦										
Tsz Tin Road I	EB	LT	B1	4	3.38	10.0		100	1698	143	0.084	0.084	100	1698	98	0.058	0.058
		LT+SA+RT	B2	4	3.37	15.0		98	1905	161	0.085	0.00	94	2092	120	0.057	0.00
<u> </u>			ا ا	 	<u> </u>	\mid	<u> </u>	 '		<u> </u>							
ļ			ا ا	—		 	└── '	 '		 							
				──	—	—	<u> '</u>	 '		—							└───┦
				├──	──	──	\vdash	 '		├							├───┦
			[]	├──	┼──		├'	 '									├───┦
				<u> </u>	+		┝──┦	\vdash					┢──┤				
pedestrian pha	ase		E _(P)	2		min c	rossing	time =	5	sec	GM +	10	sec F	GM =	15	sec	
			F _(P)	3		min c	rossing	time =	5	sec	GM +	6	sec F	GM =	11	sec	
			 	Ē	<u> </u>	ļ			[ļ			<u> </u>			[
			ا ا	──				I									
				──	──	──				──		!				 	
			[]	├	┼──	├				<u> </u>						<u> </u>	
*** T#:- Elow (pou/b				DM Troffic	(pou/br					<u> </u>			L		A Later		
AM Traffic How (pcum	r)		N	PM frame	HOW (pcu/m)				N	S = 1940 +	⊦ 100 (W-3.:	25) S =	= 2080 + 10	0 (W-3.25)	Note:		
21	° 241 a	. 21			16/	207	. .	20		SM = S / (1 + 1.5 f/r)	SM =	(S - 230) / ((1 + 1.5 f/r)			
	6 341 4 ►3	→ 21	'			7		39			AM	Peak	PM	Peak			
↓ 8'	5	465 46	İ		↓ 47	,	298	39		C.m.v	1+2+3+4 0 420		1+2+3+4				
	198 ▲	7	I			212 •	7	+		L (s)	24		24				
4	3 + 7	↓ 14	İ		40	· + + •	17	↓ 12		C (s)	118		118				
	I		İ			1				practical y	0.717		0.717				
										R.C. (%)	67%		111%				
1	A2 A1	2				3				4				5			
							•				t						
F _(P) ↓	↓ ↓	F _(P)	Ť +			F _(P)	î ↓	ŧ		B1 B2	\rightarrow						
			┤ऻ॓┌╸	•				\leftarrow	D1		÷						
			C1 C2 C3							E _(P)	ŧ						
						<u> </u>				<u> </u>							
AM G =	=	I/G = 6	G =		I/G =	6	G =		I/G =	9	G =		I/G =	7	G =		
G =	-	I/G = 0	G =		I/G =	6	G =		I/G =	9 0	G =	14	I/G =	7	G =		
PM G=	-	1/G = 0	G =		//G =	6	G =		/G =	9	G =	14	i/G =	7	G =		
	-	10 - 0				<u> </u>				<u> </u>		<u> </u>	10-				

						Signal J	unction A	nalysis									
Junction:	Tsing Lur	n Road/ Tsz Tir	n Road												Job Nu	mber:	J7265
Scenario:	Without D	Development														<u> </u>	26
Design Year:	2033	Designe	əd By:				-	Checke	d By:				-	Date:	21	1 Feb 20)24
					,	,				AM Peak					PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)	/ % Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Tsing Lun Roa	d SB	LT+SA	A1	1	3.65	13.0		4	1971	753	0.382	0.382	11	1955	482	0.247	0.247
		RT	A2	1	3.65	15.0	'	100	1927	567	0.294	ļ'	100	2120	491	0.232	ļ!
			—				'	 '	└── ′	—	—	'	—			_	
			\vdash	\vdash		<u> </u>	'	<u> </u> '		<u> </u>			\vdash	<u> </u>			
		SA	C1	2	3.30	23.0	+'	53	1880	189	0.101	0.101	66	1865	225	0.121	0.121
		RT	02	2	3.40	15.0	+	100	1005	10	0.005	+'	100	1005	255	0.013	
			0.5		0.40	10.0	+	100	1903	10	0.005		100	1803	23	0.015	
Access Road t	0	LT+SA+RT	D1	3	3.50	15.0	1	88	1806	85	0.047	0.047	87	1808	75	0.041	0.041
Siu Hong Cour	t WB																
Tsz Tin Road E	ΞB	LT	B1	4,1	3.38	10.0	'	100	1698	481	0.283	ļ!	100	1698	290	0.171	ļ!
 		SA+RT	B2	4	3.37	15.0	'	90	1919	100	0.052	0.052	88	1923	73	0.038	0.038
 			├──	──	──	──	<u> </u>	 '	<u> </u> '	──	──	'	──	──		──	──┦
			├	──	──	──	<u> </u>	 '	├ ──'	──	──	'	──	──	──	──	──┦
			<u> </u>					 '					┣──				├ ──┦
			<u> </u>				+'	╞───┘					├──				
							<u> </u>						<u> </u>				
													 				
pedestrian pha	ise		E _(P)	2		min c	crossing	time =	5	sec	GM +	10	sec F	-GM =	15	sec	
			F _(P)	3	<u> </u>	min c	crossing	time =	5	sec	GM +	6	sec F	-GM =	11	sec	<u> </u>
			 	<u> </u>					└── '	<u> </u>		<u> </u>	<u> </u>		<u> </u>	<u> </u>	
			──		──	──			└── ′								
			├──	──	──	──			├ ─── [′]	──		──	──		───	──	
			├──	──	──	──			<u> </u> '	──		──	──		<u> </u>	──	
			├──	├──	┼───	┼───			<u>├</u> ──'	├──			├──		├──	├──	
** 1 T#:- Elow (pou/b	``		<u> </u>	Dad Troffic		<u> </u>						<u> </u>					
AM Traffic How (pourni)		Ν	РМ Папс	Flow (pcu/ni)	1			Ν	S = 1940 →	+ 100 (W-3.	.25) S :	= 2080 + 10)0 (W-3.25)	Note:		
		1								SM = S / (*	1 + 1.5 f/r)	SM =	(S - 230) /	(1 + 1.5 f/r)	4		ļ
	1 567 🗲	→ 30	I		290	491	$ \rightarrow $	51	'		AM	Peak	PM	Peak	4		
↓ j	10	723 55			, •	9 1	431	45	I	┣──	1+2+3+4	──	1+2+3+4	──	-		
	298	10			-	329	10	, 	. !	Sum y	0.582		0.447		ł		
10 [.]	1 + 10	0 20			149	, ---	• 25	↓ 20	ł	L (s)	118		118		1		
	I	20				I		20	I	oractical v	0 717		0 717				İ
									I	R.C. (%)	23%	<u> </u>	61%		1		
1			_			2			_	1				5			
1	A2 A1	Ĺ				3			I	4				5			
_{В1} т	┥┟╸	F _(P)				F _{(P}	▲ 1 1 ±		I	B1 B2							
	•		* 11 r	•			*	+	D1	D2	- + ·						
			411					ŧ	I	E _(P)	 ↓						
			C1 C2 C3														
AM G =		I/G = 6	G =		I/G =	6	G =		I/G =	9	G =		I/G =	7	G =		
G =		I/G = 6	G =		I/G =	6	G =		I/G =	9	G =		I/G =		G =		
PM G =		I/G = 6	G =		I/G =	6	G =		I/G =	9	G =		I/G =	7	G =		
G =		I/G = 6	G =		I/G =	6	G =		I/G =	9	G =		I/G =		G =		

						Signal J	unction A	nalysis									
Junction:	Tsing Lun	Road/ Tsz Tir	n Road												Job Nu	mber:	J7265
Scenario:	With Appr	roved Scheme	<u>;</u>													<u>P.</u>	27
Design Year:	2033	Designe	ed By:				-	Checke	d By:				-	Date:	2*	1 Feb 20	24
					T	T	T			AM Peak			<u> </u>		PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Tsing Lun Roa	.d SB	LT+SA	A1	1	3.65	13.0		4	1971	761	0.386	0.386	10	1957	484	0.247	0.247
		RT	A2	1	3.65	15.0	''	100	1927	567	0.294	<u> '</u>	100	2120	491	0.232	<u> </u> !
			—				·	 '	└── ′	—	_	<u> '</u>	_	_			\mid
		17+64	<u> </u>	<u> </u>	<u> </u>	<u> </u>	'	<u> </u> '	<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>	\vdash	<u> </u>	<u> </u>
TSING LUN NOA		SA	C1	2	3.30	23.0	<u> </u>	53	1880	190	0.101	0.101	66	1865	226	0.121	0.121
			02	2	3.40	45.0	<u> </u> '	100	2095	10	0.101		100	2095	254	0.121	├
			US .	2	3.40	15.0		100	1905	10	0.005		100	1905	25	0.013	├──┦
Access Road t	10	LT+SA+RT	D1	3	3.50	15.0		88	1806	85	0.047	0.047	87	1808	75	0.041	0.041
Siu Hong Cour	rt WB		<u> </u>		0.00	10.0			1000		0.01.	0.0	<u> </u>	1000		0.01.	0.0
Tsz Tin Road I	ĒB	LT	B1	4,1	3.38	10.0		100	1698	481	0.283		100	1698	290	0.171	
		SA+RT	B2	4	3.37	15.0		90	1919	100	0.052	0.052	88	1923	73	0.038	0.038
			\vdash	<u> </u>	\vdash	\vdash	ļ'	 '	 '	<u> </u>	<u> </u>	<u> </u> '		<u> </u>	<u> </u>	<u> </u>	<u> </u>
]					ļ'	 '	└── ′			↓ '	_				ļ!
			—		—	—	'	 '	└── ′			<u> '</u>	—		──	──	\vdash
			—		┼───	┼───	'	 '	<u> </u> '			───′	┣───		──	──	──┦
			├──		┼──	┼──	<u> </u>	 '	├ ─── [′]			──′	┨────		┼───	┼───	┼───┦
			├──	├──	┼───	┼───	'	 '	<u> </u> '			──′	├──		┼───	┼───	┝───┤
			├──		<u> </u>	<u> </u>	+	┟───┘	\vdash			┼───┦			<u> </u>	<u> </u>	
pedestrian pha	ase		E _(P)	2		min c	rossing	time =	5	sec	GM +	10	sec F	-GM =	15	sec	
, person (=,, , F _(P)	3		min (rossing	time =	5	sec	GM +	6	sec F	GM =	11	sec	
							100.0			-				<u></u>			
			\vdash	<u> </u>	\vdash	\vdash			 '	<u> </u>		ļ'	\vdash		\vdash	\vdash	<u> </u>
					──	──			└── ′			' ا			──	──	
					L	L									L	L	
AM Traffic Flow (pcu/h	0		Ν	PM Traffic	Flow (pcu/hr)	j			Ν	S = 1940 ·	+ 100 (W-3.	.25) S -	= 2080 + 10)0 (W-3.25)	Note:)		
		I					I			SM = S / (1 + 1.5 f/r)	SM =	(S - 230) /	(1 + 1.5 f/r)	1		ļ
t ⁴⁸	1 567 🔶	→ 30	1		1 290	491	↔	51	l I	ſ	AM	Peak	PM	Peak]		
\rightarrow	10	♦ 731 55			\rightarrow	9	4 33	45	ł	<u> </u>	1+2+3+4	''	1+2+3+4	 	4		
90	300	. 1			64	331		1	I	Sum y	0.586		0.448	<u> </u>	4		
10		10			1.46	, Î.	10	+	ļ	L (s)	24	'	24	──	-		
10		20			143	\square	25	20	ŀ	C (s)	118	'	118	┣───	4		
									I	practical y	0.717	'	0.717	┼───	1		
				<u> </u>						R.C. (%)	22%		60%	<u> </u>	<u> </u>		
1	A2 A1	2				3			I	4				5			
в1	┛┝		↑				≜		I	B1	t						
5.	~ +	F _(P)	÷			F _(P)	÷	. †		B2							
		1	┥║	*				-	D1	Ea	ŧ						
			C1 C2 C3	i					I	(P)	ŧ						
														7			
AM G=		1/G = 0	G≃ G=		//G =	6	G =		⊮G =	9 Q	G≡		I/G =	1	G =		
PM G=	_	I/G = 6	 G =		I/G =	6	G =		I/G =	9	G =		I/G =	7	G =		
					1/0	6	<u> </u>		I/G =	٩	6 -		I/G =		G =		

						Signal J	unction A	nalysis									
Junction:	Tsing Lur	n Road/ Tsz Ti	n Road												Job Nu	mber:	J7265
Scenario:	With Prop	posed Scheme	<u>.</u>													<u> </u>	28
Design Year:	2033	Designe	əd By:				-	Checke	ed By:				•	Date:	21	i Feb 20)24
	·					(m	T		C . Show	AM Peak	har	a or also		C . Show	PM Peak	har	a Xi aliy
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hiii Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Tsing Lun Roa	id SB	LT+SA	A1	1	3.65	13.0	'	4	1971	776	0.394	0.394	10	1957	487	0.249	0.249
		RI	A2	1	3.65	15.0		100	1927	567	0.294		100	2120	491	0.232	$\left - \right $
Tsing Lun Roa	d NB	LT+SA	C1	2	3.30	23.0	'	53	1880	191	0.102	0.102	65	1866	228	0.122	0.122
		SA RT	C2	2	3.40		'		2095	213	0.102	┝──┤		2095	256	0.122	
			C3	2	3.40	15.0		100	1905	10	0.005		100	1905	25	0.013	┼──┦
Access Road t	0	LT+SA+RT	D1	3	3 50	15.0		88	1806	85	0.047	0.047	87	1808	75	0.041	0.041
Siu Hong Cour	t WB				0.00				1000		0.0	0.0	<u> </u>	1000		0.01.	0.011
Tsz Tin Road f	EB	LT	B1	4.1	3.38	10.0		100	1698	481	0 283		100	1698	290	0 171	$\left - \right $
		SA+RT	B2	4	3.37	15.0		90	1919	100	0.052	0.052	88	1923	73	0.038	0.038
				<u> </u>	—	<u> </u>									<u> </u>		┝──┦
			—		<u> </u>	 		 									\mid
			<u> </u>	├──	┼───							├ ──┤					
												╞──┦					
pedestrian pha	ise		E _(P)	2		min c	rossing	time =	5	sec	GM +	10	sec F	GM =	15	sec	
			F _(P)	3	──	min c	rossing	time =	5	sec	GM +	6	sec F	GM =	11	sec	┨───┤
			<u> </u>														
						<u> </u>										t	
			 	──	──	├──						'				 	
AM Traffic Flow (pcu/h	r)		N	PM Traffic	Flow (pcu/hi	·)			N	I T					Note:		
		I								S = 1940 + SM = S / (+ 100 (W-3. 1 + 1.5 f/r)	.25) S =	: 2080 + 10 (S - 230) / /	0 (W-3.25) (1 + 1.5 f/r)			
1 ⁴⁸¹	1 567 🗲	→ 30			t ²⁹⁰	491	.↔	51			AM	Peak	PM	Peak			
	10	↓ 746			\rightarrow	9	↓ 436	45			1+2+3+4		1+2+3+4				
. 90) 303	55			64	335		45		Sum y	0.594		0.450	<u> </u>			
10				1	140		10	+		L (s)	24	i	24				
101		20		1	145	•	25	20		C (s)	118		118		•		
				1						practical y	0./1/		0.717				
				<u> </u>						K.U. (70)	2170		3375	l r	I		
1	A2 A1	2				3				4				5			
в1 т	┛┝	F _(P)	≜			F _{(P}	▲ 1 1			B1 B2			İ				
			* ††г	*			•	+	· D1	52	+		İ				
			H_{III}					+		E _(P)	↑ ↓		l				
			C1 C2 C3														
AM G =		I/G = 6	G =		I/G =	6 6	G =		I/G =	9 9	G =		I/G =	7	G =		
PM G =		I/G = 6	G =		I/G =	6	G =		I/G =	9	G =		I/G =	7	G =		
G =		I/G = 6	G =		I/G =	6	G =		I/G =	9	G =		I/G =		G =		

Junction:		San Hing	Road / N	g Lau Ro	ad (Southe	ern)				
Design Y	ear:	2023		Job Numl	oer: J	7265	Da	te:	21 Feb	2024
Scenario	:	Existing C	Condition							P. 29
-	Ng Lau I	Road (Arm	C)					Ng L	au Road	(Arm A)
	<u>152</u>	166		→						
	<u>1</u>	7		<u>_</u>					<u> </u>	
				· ←_			•	— L	93	<u>214</u>
									24	<u>18</u>
							*	-		
				4	21				AM	<u>PM</u>
		North	l	0	<u>15</u>	-				
				San Hir	ig Road (A	rm B)				
The pred Q-BA = Q-BC = Q-CB = The geor D = E = F = where	ictive equ D[627 + E[745 - F[745 -	uations of c 14W-CR Y(0.364q-/ 0.364Y(q-/ rameters re 94(w-BA - 94(w-BC - 94(w-CB - .0345W c = the des or road wid central res c = lane wit c = visibilit c = visibilit	capacity c Y(0.364 AC + 0.14 AC + q-AE epresente 3.65)][1 + 3.65)][1 + 3.65)][1 + ign flow of th serve width idth to vel y to the re- y to the left	of moveme q-AC + 0. [4q-AB]] B)] ed by D, E - 0.0009(\ - 0.0009(\ - 0.0009(\ of movement th hicle ight for wait	ent are: 144q-AB + /-rBA - 120 /-rBC - 120 /-rCB - 120 ent AB, etc aiting vehicle	0.229q-))][1 + 0.())]))] ; eles in stress in stress	CA + 0.52q- 0006(V-IBA - eam BA, etc am BA, etc	CB)] - 150)]		
Geometr	v:		Inc	out	Inpu	ıt	Input		Calcul	ated
	,		W	6.65	V-rBA	23	w-BA	2.40	D	0.7441
			W-CR	0.00	V-IBA	23	w-BC	2.40	Е	0.8078
					V-rBC	26	w-CB	3.00	F	0.8857
					V-rCB	57			Y	0.7706
Analysis	:					_				
Traffie	c Flows,	pcu/hr	AM	PM		Cap	acity, pcu/hr		AM	PM
	q-CA		166 -	152		(Q-BA		421	400
	d-CR		1	1		(M-RC		5/9	552 602
	q-AB q-AC		24 02	18 217					031 110	00∠ 400
	q-AC a-BA		93 21	∠14 15					440	400
	g-BA		۲ <u>۲</u>	10						
	q-DC f		0 160	0 000						
			5.100	0.000			514			
			Ratio-of	-tlow to C	apacity	AM	PM			
				в-A в C		0.050	0.037			
						0.007	0.000			
				B-AC		0.011	0.002			
				27.0		0.007	0.001			















Junction:		T-junction	of San H	ing Road						
Design Yea	ar:	2023		Job Numl	oer: .	17265	Da	te:	21 Feb	2024
Scenario:		Existing C	Condition		_					P. 37
S	San Hing	Road (Ar	m C)					San Hi	ng Road	(Arm A)
	<u>15</u>	23		→						
	1	1		Ъ						
				★			+	— T	24	17
									8	2
				0	3				AM	PM
N	North ·	←──		0	0					
				Unr	amed Roa	ad	•			
The predic	tive eau	ations of o	capacity o	f moveme	ent are:					
Q-BA = [D[627 +	14W-CR ·	Y(0.364c	-AC + 0.	144a-AB +	- 0.229a-	CA + 0.52a-	CB)1		
Q-BC = B	E[745 - \	Y(0.364a-	AC + 0.14	4a-AB)1		1	1	- /]		
Q-CB = B	=[1 · · · F[745 - () 364Y(a-A	$AC + \alpha - AB$)]						
The geome	etric par	ameters re	epresente	d by D F	F are:					
D – [[1 + 0 0C	4/w-BA -	3 65)][1 +		/-rRA - 12()][1 + 0 (006(V-IBA	. 150)]		
F – [[1 + 0.00 [1 + 0.00	4(w-BC -	3 65)][1 +	0.0000(\	/-rBC - 12	0)][0.(0)]	0000(112/1	100/]		
	[1 + 0.00 [1 + 0.00	A(w-CB -	3 65)][1 +		/_rCB _ 12	0)]				
where V	/ _ 1 _ 0	0345\\\	0.00)][11	0.0000(1	100 12	0)]				
	-1-0.	- tho doc	ian flow o	fmovom	ont AR of	~				
Ч У	V = mai	r rood wie	Nyn now o Nh	movern		0				
V		ontrol roc	uu oraa widt	h						
V				() Viele						
W		; = lane wi								
V	-rba, et		y to the h	gnt for wa	atting venic	cies in str	eam BA, etc	;		
V	'-IBA, etc		y to the le	it for wait	ing venicle	es in strea	am BA, etc			
Comptant			المعا		اسما	.4	اسمر		Calau	lata d
Geometry	:		Inp			JT		0.50	Calcu	
				6.00	V-IBA	25	W-BA	2.50	D	0.7912
			W-CR	0.00	V-IBA	100	W-BC	2.50	E	0.8156
					V-rBC	25	w-CB	3.00	F	0.8586
					V-rCB	25			Y	0.7930
Analysis :						-				
Traffic	Flows, p	ocu/hr	AM	PM		Cap	acity, pcu/hr		AM	PM
q	-CA		23	15		(Q-BA		486	490
q	I-CB		1	1		(Q-BC		601	603
q	I-AB		8	2		(Q-CB		632	635
q	I-AC		24	17		(Q-BAC		486	490
q	-ВА		3	0						
q	I-BC		0	0						
f			0.000	0.000						
			Ratio-of-	flow to C	apacity	AM	PM			
			E	3-A	-	0.006	0.000			
			E	3-C		0.000	0.000			
			(С-В		0.002	0.002			
			E	B-AC		0.006	0.000			







Junction:	Ng Lau Road	/ Castle P	eak Roa	ad - Larr	n Tei										Job Nu	mber:	J7265
Scenario:	Existing Cond	dition														Ρ.	41
Design Year:	2023	Designe	ed By:					Checke	d By:					Date:	21	Feb 20	24
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turnina %	Sat. Flow	AM Peak Flow	v value	Critical v	Turnina %	Sat. Flow	PM Peak Flow	v value	Critical v
Na Lau Dood ER		1.7	A 1	0	4.00	20.0	Gradient	100	(pcu/hr)	(pcu/hr)	0.057	0.057	100	(pcu/hr)	(pcu/hr)	0.071	0.071
Ng Lau Road EB		LI	AI	2	4.00	20.0		100	10/4	107	0.057	0.057	100	10/4	134	0.071	0.071
Castle Peak Roa	d - Lam Tei NB	LT	B1	1	3.50	15.0		100	1786	130	0.073	0.073	100	1786	128	0.072	0.072
															-		
nedestrian pha	160		Car	12		min.c	rossing	time –	8	8001	⊇M +	8	soc F	GM -	16	800	
pedestilari priz	130		D _(P)	3		min c	rossing	time =	7	sec	GM +	7	sec F	GM =	14	sec	
			- (F)				rooonig .							0		000	
AM Traffic Flow (pcu/h)		N	PM Traffic I	Flow (pcu/hr)				N	S = 1940 +	- 100 (W-3.	25) S :	= 2080 + 10	0 (W-3.25)	Note:		
	107						134			SM = S / (1 + 1.5 f/r)	SM =	(S - 230) /	(1 + 1.5 f/r)			
						t	L,				AM	Peak	PM	Peak			
13					128						1 + 2		1+2				
										Sum y	0.130		0.143				
										L (s)	2358 3600		3627		ł		
										C (S)	0.312		0.333				
										R.C. (%)	140%		133%				
1		2				3				4				5			
C _(P) ⊥	C(P)	C _(P)	t.	Ą1	C _(P)		D _(P)										
· •	•		•				<≯ ++++≯	E _(LRT)									
в1							•++++										
AM G =	: I/G	i =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
G =	I/G) =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G =	I/G	i =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
G =	· I/G) =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

Junction:	Ng Lau Road	/ Castle Pe	eak Roa	ad - Lam	<u>ı Tei</u>									-	Job Nur	mber:	J7265
Scenario:	Without Deve	lopment						<u> </u>								<u> </u>	42
Design Year:	2033	Designe	∋d By:				-	Checke	d By:				-	Date:	21	Feb 20	24
				<u> </u>	T	Τ	<u> </u>			AM Peak					PM Peak	<u> </u>	
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Ng Lau Road EB	1	LT	A1	1	4.00	15.0	<u>['</u>	100	1832	137	0.075	Ĺ'	100	1832	155	0.085	Ĺ
Castle Peak Road	.d - Lam Tei NB	SA	B1	2,3	3.30	<u>ا</u>	<u> '</u>		1945	270	0.139	Ļ'	 '	1945	540	0.278	
		SA	B2	2,3	3.30	''	<u> '</u>	<u> </u>	2085	290	0.139	<u> '</u>	 '	2085	579	0.278	
		SA	B3	2,3	3.30	<u> </u>	<u> '</u>	<u> </u>	2085	290	0.139	<u> '</u>	 '	2085	580	0.278	
		RT	C1	1	3.40	15.0	<u> '</u>	100	1905	250	0.131	0.131	100	1905	182	0.096	0.096
Castle Peak Roa	d - Lam Tei NB	LT	D1	2	3.50	13.0	<u>'</u> ا	100	1762	158	0.090	↓ '	100	1762	145	0.082	↓ !
		LT	D2	2	3.40	15.0	<u> </u>	100	1905	148	0.078	<u> '</u>	100	1905	177	0.093	↓ !
		SA	E1	2,3	3.65	<u> </u>	<u>'</u> ا	└ ──'	2120	346	0.163	↓ '	 '	2095	779	0.372	0.372
		SA	E2	2,3	3.65	<u> </u>	↓ ′	↓ '	2120	346	0.163	 '	 '	2120	789	0.372	ļ!
Castle Peak Road	d - Lam Tei SB	LT+SA	F1	2,3	3.70	15.0	<u>'</u> ا	2	1981	594	0.300	0.300	4	1977	412	0.208	Ļ!
		SA	F2	2,3	3.70	↓ '	↓'	↓ '	2125	637	0.300	↓'	 '	2125	443	0.208	Ļ!
		SA	F3	2,3	3.70	<u> </u> '	↓ ′	'	2125	637	0.300	↓'	 '	2125	443	0.208	Ļ!
Castle Peak Road	d - Lam Tei SB	LT+SA	G1	1,2,3	3.65	10.0	<u> </u>	11	1948	754	0.387	<u> </u>	26	1906	459	0.241	\vdash
		SA	G2	1,2,3	3.65	<u> </u>	<u> </u>	<u> </u>	2120	821	0.387	<u> '</u>	 '	2120	511	0.241	└── ノ
		RT	H1	4	3.50	20.0	<u>'</u> ا	100	1958	101	0.052	0.052	100	1958	82	0.042	0.042
Wong Kong Wai	Road WB	LT	1	4	4.00	10.0	<u> '</u>	100	1752	20	0.011	<u> '</u>	100	1752	14	0.008	\vdash
Fuk Hang Tsuen	Road WB	LT	J1	5	3.65	12.0	<u> '</u>	100	1760	274	0.156	0.156	100	1760	241	0.137	0.137
		LT+RT	J2	5	3.65	15.0	<u> '</u>	100	1927	301	0.156	<u> '</u>	100	1927	264	0.137	\sqcup
Road P1		LT+RT	K1	5	3.80	12.0	<u> '</u>	100	1773	183	0.103	Ļ'	100	1773	197	0.111	
		RT	K2	5	3.80	15.0	<u>'</u> '	100	1941	200	0.103	Ĺ'	100	1941	215	0.111	Ĺ
			Ĺ'	[<u> </u>	<u>['</u>	<u>['</u>	Ĺ	<u>[</u> '	['	<u>['</u>	<u>['</u>	<u>[</u> '	<u>[</u> '	<u>['</u>	<u>[</u> !
pedestrian pha	ase		L _(P)	3,4,5		min c	rossing f	time =	5	sec (GM +	11	sec F	-GM =	16	sec	
			M _(P)	3		min c	rossing	time =	5	sec (GM +	15	sec F	-GM =	20	sec	
			N _(P)	3,4		min c	rossing	time =	5	sec (GM +	16	sec F	GM =	21	sec	<u>[</u> !
			O _(P)	5		min c	rossing	time =	5	sec (GM +	14	sec F	-GM =	19	sec	
			P _(P)	4		min c	rossing	time =	5	sec (GM +	18	sec F	-GM =	23	sec	
			Q _(P)	3		min c	rossing	time =	11	sec (GM +	11	sec F	GM =	22	sec	
			R _(P)	1,2,4,5	,	min c	rossing	time =	5	sec (GM +	13	sec F	-GM =	18	sec	
			'						'		'					<u> </u>	
AM Traffic Flow (pcu/hr	n)		N	PM Traffic	Flow (pcu/hr	()			N						Note:		_
	137 2	^{روه} 101	Ì'	1		155	, 330	82	Ì'	S = 1940 +	- 100 (W-3.2	25) S =	= 2080 + 10	.0 (W-3.25)	1		İ
	4	<i>IL</i>		1		L,	· .	4 -		SM = 57 (1 + 1.5 t/r)	Sivi =	(S - 230) / ((1 + 1.5 m)	4		İ
850	158 148		ļ	1699	`	145 177			ļ	1	AM	Peak	PM	Peak	1		
250	692		ļ		,		,		ļ	\vdash	1+2,3+4+5	<u> </u> '	1+2,3+4+5	'	ł		
1 200		101	P					82	P	Sum y	0.638	<u> </u> '	0.640	├ ───'	1		
		1565		1				955 2	. '	L (s)	120	├ ───'	100	 '	1		
-	1787	↓ ↓	ļ	1	+	∏ ☐		ار	ļ	C (s)	120	<u> </u> '	120	'	1		
429 to 146	• 20]	1	312	197		1 4	15	1	practical y	0.743	 '	0.743	 '	1		
423			<u> </u>						<u> </u>	R.C. (%)	16%	<u> </u>	15%	<u> </u>	<u> </u>		
1 R _(P)	A1 R _(P)	2	R _(P)		R _(P) ↓	3			Q _(P)	4	R _(P)	L _(P)	R _(P)	5	R _(P)	L _(P)	K1 K2 R(P)
	l ,	B1 →	D1			в1 →	← ┼ - 	- 	I		*		I			*	44
		$B2 \longrightarrow B3 \longrightarrow C$				$B2 \longrightarrow B3 \longrightarrow$	E1		I	1	M(P)		I				
C1 🚽		· ·	E2	• F3				· F3			N _(P)		₫н1		N _(P)		
	.	32 G1	÷	F2 • F1		1		F2 • F1		P _(P) ← - I	• *	•1	I	_{•1•1} •	• *	O _(P)	•
	•		<u> </u>		•		· · ·		• ·			I 1		I I J1 J2	<u>.</u>		_
AM G =	I/G :	= 8	G =		I/G =	7	G =		I/G =		G =		I/G =	5	G =		I/G = 5
G =	I/G :	=	G =		I/G =		G =		I/G =		G =		I/G =		G =		I/G =
PM G =	I/G :	= 8	G =		I/G =	7	G =		I/G =		G =		I/G =	5	G =		I/G = 5
G =	I/G ·	=	G =		I/G =		G =		I/G =		G =		I/G =		G =		I/G =

·								-									
Junction:	Ng Lau Road	/ Castle P	eak Roa	<u>ad - Lam</u>	ı Tei									-	Job Nu	mber:	J7265
Scenario:	With Approve	<u>d Scheme</u>	·													<u> </u>	43
Design Year:	2033	Designe	ed By:				-	Checke	d By:					Date:	21	Feb 20	24
		<u> </u>			<u> </u>	Π				AM Peak					PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Ng Lau Road EB	<i>i</i>	LT	A1	1	4.00	15.0		100	1832	137	0.075		100	1832	155	0.085	
Castle Peak Roa	d - Lam Tei NB	SA	B1	2,3	3.30				1945	270	0.139			1945	540	0.278	
		SA	B2	2,3	3.30				2085	290	0.139			2085	579	0.278	
		SA	B3	2,3	3.30				2085	290	0.139	[]		2085	580	0.278	Ī
		RT	C1	1	3.50	15.0		100	1914	250	0.131	0.131	100	1914	182	0.095	0.095
Castle Peak Roa	ıd - Lam Tei NB	LT	D1	2	3.50	13.0		100	1762	158	0.090		100	1762	145	0.082	
<u> </u>		LT	D2	2	3.40	15.0	[!	100	1905	148	0.078		100	1905	177	0.093	
		SA	E1	2,3	3.65				2120	346	0.163			2095	779	0.372	
		SA	E2	2,3	3.65				2120	346	0.163			2120	789	0.372	0.372
Castle Peak Roa	ıd - Lam Tei SB	LT+SA	F1	2,3	3.70	15.0		2	1981	594	0.300		4	1977	412	0.208	
		SA	F2	2,3	3.70				2125	637	0.300			2125	443	0.208	
		SA	F3	2,3	3.70	\Box			2125	637	0.300	0.300		2125	443	0.208	
Castle Peak Roa	ıd - Lam Tei SB	LT+SA	G1	1,2,3	3.65	10.0	<u> </u>	11	1948	754	0.387		26	1906	459	0.241	
		SA	G2	1,2,3	3.65		<u> </u>		2120	82 <u>1</u>	0.387			2120	511	0.241	
		RT	H1	4	3.50	20.0		100	1958	101	0.052	0.052	100	1958	82	0.042	0.042
Wong Kong Wai	Road WB	LT	1	4	4.00	10.0		100	1752	20	0.011	<u>ц</u> .	100	1752	14	0.008	
Fuk Hang Tsuen	Road WB	LT	J1	5	3.65	12.0		100	1760	274	0.156	0.156	100	1760	241	0.137	0.137
		LT+RT	J2	5	3.65	15.0		100	1927	301	0.156	0.12	100	1927	264	0.137	0
Road P1		LT+RT	K1	5	3.80	12.0		100	1773	183	0.103		100	1773	197	0 111	
		RT	K2	5	3.80	15.0		100	1941	200	0.102		100	1941	215	0.111	
				<u> </u>	-	+		100	107.	200	0.100		100	10-1.	210	0.11.	
pedestrian pha	200		I.(p)	3.4.5	<u> </u>	min c	rossing	time =	5	sec	<u>см</u> +	11	sec F	GM =	16	sec	
peucou	150		<u>с</u> (г) М _(Р)	3		min c	rossing	time =	5	sec	CM +	15	Sec F	-GM =	20	Sec	
1			N _(P)	34		min c	rossing	time =	5	sec	<u>∽</u> M +	16	Sec F	-GM =	21	Sec	
1			0(P)	5		min c	rossing	time =	5	sec	CM +	14	Sec F	-GM =	19	Sec	
ł			P.(P)	4	 	min c	rossing	time =	5	Sec		18	Sec. F	-GM =	23	500 660	
ł			(P)	3	<u> </u>	min c	rossing	time =	11	Sec	<u>-</u> M+	11	Sec. F	-GW =	20	900 900	
			R(P)	124.5	.	min c	rossing		5	Sec	<u>-</u> M+	13	Sec. F	-GW =	18	900 900	
			rx(P)	1,2,7,5	 	11111.5	ໃບຮວກາງ -		U U	300	JIVI T	10	560.		10	১৮৬	├
			·		<u> </u>								<u> </u>				<u> </u>
AM Traffic Flow (pcu/nr	r) 		N	PM Traffic H	Flow (pcu/hr))			N	S = 1940 +	+ 100 (W-3.:	25) S =	= 2080 + 10	00 (W-3.25)	Note:		
1	137 2	²⁸² // ¹⁰¹		1		155	330	// 82		SM = S / (*	1 + 1.5 f/r)	SM =	(S - 230) / ((1 + 1.5 f/r)	l		
1	158 ÷	14	1 1	1		145	• •	4	I		AM	Peak	PM	Peak			
850	¹⁴⁸		I	1699	j.	<u> </u>					1+2,3+4+5		1+2,3+4+5	[]		
★ 250			I	▶ 182	2	1568				Sum y	0.638		0.646		1		
1		101	I	1				82		L (s)	21		21		1		
•	1787	1565 🚄	I	1	+	1177		955 _		C (s)	120	Γ	120	Γ]		
▲ 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	√ 81	+ 10	I	₊	•		• ⊓	↓ 15		practical y	0.743		0.743	Γ	1		
429 14	6 20			313	, 192	2	14			R.C. (%)	16%		15%		1		
. Bai	^1 R		P_1		P., I	 ا		⊢,-▶ ·	₹- ►	а <u>вана.</u> Т.	î <	-,-▶	D.,1	Г <u>.</u>		-,->	*** 140 R.J.
1 ^r ∖(₽), ▼		* ²	^K (P)↓		^{R(P)} ↓	3	++ +		Q _(P)	4	^K (P)↓	L _(P)	^K (P)↓	5	^r ^(P) ↓	L _(P)	
1		$B1 \longrightarrow B2 \longrightarrow$				$B1 \longrightarrow B2 \longrightarrow$					†		İ				
C1 -		вз→	$E_1 \longrightarrow E_2 \longrightarrow$			вз→	$E1 \longrightarrow E2 \longrightarrow$				↓ ^{IVI(P)}		⊀		•		
,		G2	Ì ↓	F3 F2	← G:	2	• •	• F3 • F2	← G2	P(P)	N _(P)		∠н		N _(P) ↓	O _(P)	
1	↓	G1	*+	F1	↓ G1	1	*∓	F1	↓ G1	←-	•	1	İ	│ ¶♥	•	← -)	
		<u> </u>										I1		J1 J2			
AM G =	I/G :	= 8	G =		I/G =	7	G =		I/G =		G =		I/G =	5	G =		I/G = 5
G =	I/G :	=	G =		I/G =		G =		I/G =		G =		I/G =		G =		I/G =
PM G =	I/G :	= 8	G =		I/G =	7	G =		I/G =		G =		I/G =	5	G =		I/G = 5
G =	I/G	=	G =		I/G =		G =		I/G =		G =		I/G =		G =		I/G =

Junction:	Ng Lau Road	I / Castle F	'eak Ro	ad - Lar	n Tei									-	Job Nu	mber:	J7265
Scenario:	With Propose	ed Scheme	<u>)</u>													Ρ.	44
Design Year:	2033	Designe	əd By:				-	Checke	ed By:				-	Date:	21	Feb 20	124
					Π	Τ	<u> </u>			AM Peak		<u> </u>			PM Peak	<u> </u>	
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Ng Lau Road EF	3	LT	A1	1	4.00	15.0	ļ'	100	1832	138	0.075	└── ′	100	1832	155	0.085	·
Castle Peak Roa	ad - Lam Tei NB	SA	B1	2,3	3.30		' ا	 '	1945	271	0.139	<u> </u>	 '	1945	540	0.278	<u> </u> '
		SA	B2	2,3	3.30	──	'	 '	2085	290	0.139	↓ '	 '	2085	579	0.278	 '
		SA	B3	2,3	3.30	\vdash	'	 '	2085	290	0.139	↓ '	 '	2085	580	0.278	 '
		RT	C1	1	3.50	15.0	'	100	1914	250	0.131	0.131	100	1914	182	0.095	0.095
Castle Peak Roa	ad - Lam Tei NB	L1		2	3.50	13.0	ļ'	100	1762	158	0.090	—	100	1762	145	0.082	 '
			D2	2	3.40	15.0	'	100	1905	149	0.078	↓ '	100	1905	177	0.093	 '
		SA		2,3	3.65	──	'	 '	2120	346	0.163	<u> </u> '	 '	2095	779	0.372	↓ '
		SA	E2	2,3	3.65	<u> </u>	'	 '	2120	346	0.163	<u> </u> '	 '	2120	789	0.372	0.372
Castle Peak Roa	ad - Lam Tei SB	LT+SA		2,3	3.70	15.0	'	2	1981	595	0.300	<u> </u> '	4	1977	413	0.209	 '
		SA	F2	2,3	3.70	──	'	 '	2125	638	0.300	└── ′	 '	2125	444	0.209	—┘
		SA	F3	2,3	3.70	<u> </u>	'	 '	2125	637	0.300	0.300	 '	2125	443	0.208	↓ '
Castle Peak Roa	ad - Lam Tei SB	LT+SA	G1	1,2,3	3.65	10.0	'	11	1948	755	0.388	<u> '</u>	26	1906	460	0.241	<u> </u> !
		SA	G2	1,2,3	3.65	\vdash	'	 '	2120	821	0.387	<u> '</u>	 '	2120	511	0.241	<u> </u> !
		RT	H1	4	3.50	20.0	'	100	1958	101	0.052	0.052	100	1958	82	0.042	0.042
Wong Kong Wa	i Road WB	LT		4	4.00	10.0	ļ'	100	1752	20	0.011	↓ '	100	1752	14	0.008	<u> '</u>
Fuk Hang Tsuer	1 Road WB	LT	J1	5	3.65	12.0	ļ'	100	1760	274	0.156	0.156	100	1760	241	0.137	0.137
		LT+RT	J2	5	3.65	15.0	ļ'	100	1927	301	0.156	<u> '</u>	100	1927	264	0.137	<u> '</u>
Road P1		LT+RT	K1	5	3.80	12.0	ļ'	100	1773	183	0.103	<u> '</u>	100	1773	197	0.111	<u> '</u>
<u> </u>		RT	K2	5	3.80	15.0	ļ'	100	1941	201	0.104	<u> '</u>	100	1941	216	0.111	<u> '</u>
L			 '	Ļ	<u> </u>	<u> </u>			Ļ	\vdash		<u> </u> !			<u> </u>	<u> </u>	<u>'</u> ــــــــــــــــــــــــــــــــــــ
pedestrian pha	ase		L _(P)	3,4,5	<u> </u>	min c	rossing	time =	5	sec	GM +	11	sec F	-GM =	16	sec	
<u> </u>			M _(P)	3	 	min c	rossing	time =	5	sec (GM +	15	sec F	-GM =	20	sec	'ا
			N _(P)	3,4	_	min c	rossing	time =	5	sec	GM +	16	sec F	GM =	21	sec	ا ــــــ ا
			O _(P)	5	_	min c	rossing	time =	5	sec	GM +	14	sec F	GM =	19	sec	ا ــــــ ا
<u> </u>			P _(P)	4		min c	rossing	time =	5	sec (GM +	18	sec F	-GM =	23	sec	''
		!	Q _(P)	3		min c	rossing	time =	11	sec	GM +	11	sec F	-GM =	22	sec	
		!	R _(P)	1,2,4,5	<u> </u>	min c	rossing	time =	5	sec	GM +	13	sec F	-GM =	18	sec	 '
			ـــــ ا														<u> </u>
AM Traffic Flow (pcu/ł	hr)		N	PM Traffic	Flow (pcu/hr	:)			N	c = 1940	· 100 (W-3	25) S.	- 2080 + 10	0 (W-3.25)	Note:		
	¹³⁸ 1	283// 101	'			155 	331	// 82		SM = S / ((1 + 1.5 f/r)	23, SM =	(\$ - 230) /	(1 + 1.5 f/r)	1		
	158	14				ل ه 145	اله م	4		0	AM	Peak	PM	Peak	1		
851	1 49		ļ	1699	,	¹⁷⁷ م			I		1+2,3+4+5	Cui	1+2,3+4+5	1	1		
250	692		ļ	182	<u>'</u>	1568			I	Sum y	0.638	<u> </u>	0.646		1		
ĺ		101						82		L (s)	21		21	1	1		
Ι.	1789	1566 👗	: '		+	1179	i.	956 /	. I	C. (s)	120		120	1	1		
L	₩ 81	- 10	. I	↓ ↓		121	•	↓ ₁₅	:	practical y	0.743		0.743		1		
429 14	46 20	1	ļ	313	الا ₁₉₂	2	14		I	R.C. (%)	16%		15%	-	1		
	,	 							← - ▶	T	<u> </u>	<u> </u>	~ 1				
1 R _(P)	A1 R _(P)	2	R _(P) ↓		R _(P) ↓	3	++		Q _(P)	4	R _(P) ↓	L _(P)	R _(P) ↓	5	R _(P)	L _(P)	K1 K2 R _(P)
	7	$B1 \longrightarrow B2 \longrightarrow$				$B1 \longrightarrow B2 \longrightarrow$	•				+		I				**
C1 -1		B3 →	$E1 \longrightarrow$ $F2 \longrightarrow$			B3 →	$E1 \longrightarrow F2 \longrightarrow$				↓ M _(P)		4				
•	←	62		• F3 • F2	← G;	,		• F3 • F2	← G2	Pre	N _(P)		∠H1		N _(P)	O(P)	
1	↓	G1	*∓	· F1	↓ G1	i l	•+	- F1	↓ G1	← →	• •	1	I	1	•	← →	•
 												<u>i1</u>		J1 J2	·		
AM G =	: I/G	= 8	G =		I/G =	7	G =		I/G =		G =		I/G =	5	G =		I/G = 5
G =	- I/G	<u> </u>	G =		I/G =		G =		I/G =		G =		I/G =		G =		I/G =
PM G =	: I/G	= 8	G =		I/G =	7	G =		I/G =		G =		I/G =	5	G =		I/G = 5
G =	= I/G	=	G =		I/G =		G =		I/G =		G =		I/G =		G =		I/G =



Junction:	Hong Po Ro	bad / San Hir	ng Road	1										-	Job Nu	mber:	J7265
Scenario:	Without Dev	velopment														Ρ.	46
Design Year:	2033	Designe	ed By:				-	Checke	d By:				•	Date:	2	1 Feb 20	24
					T	T	—			AM Peak					PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Hong Po Road W	/B	SA+LT	A1	1	3.75	10.0	['	11	1958	308	0.157	0.157	5	1975	372	0.188	0.188
		SA	A2	1	3.75				2130	336	0.158			2130	402	0.189	
			Ĺ														
Access Road NB	,	LT+RT	B1	2	3.50	15.0		100	1786	35	0.020	0.020	100	1786	8	0.004	0.004
Hong Po Road E	.B	SA	C1	1	3.65				1980	455	0.230	0.230		1980	282	0.142	0.142
		SA	C2	1	3.65				2120	488	0.230			2120	301	0.142	
San Hing Road S	₿	LT	D1	3	3.65	10.0		100	1722	73	0.042	0.042	100	1722	36	0.021	0.021
		LT+RT	D2	3	3.65	15.0		100	1800	77	0.043	0.2	100	1800	37	0.021	0.0.
				- č	0.00	10.0		100	1000	<u>, , , , , , , , , , , , , , , , , , , </u>	0.0.0		100	1000	<u> </u>	0.02.	
					<u> </u>				<u> </u>		+				+		
			<u> </u>				<u>├</u> ───'	\vdash	<u> </u>					<u> </u>			
			├───	<u> </u>		┼──	<u> </u> '	 '	├──	<u> </u>	┼──		┢───┘	<u> </u>	┼──	┼──	
		I	├──	──	┼──	┼──	'	 '	──	──	┼──		┢───┘		┼──	┼──	
			├	──	┼───	──	'	 '	├───	──	──	l	'	──	──	──	
			──	──	──	──	 '	 '	──	──	──	'	 '	──	──	──	
			┝───		—	—	'	 '	──		—	'	 '	──	—	—	
]	└──			—	ļ'	 '			—	ļ'	 '		—	—	<u> </u>
			L	<u> </u>	\vdash	\vdash	ļ'	<u> </u> '	L	<u> </u>	\vdash		<u> </u>	\square	\vdash	\vdash	<u> </u>
			L														
[Ī				['	['	Ĺ				['				
pedestrian pha	ise		E _(P)	4		min c	rossing	time =	8	sec	GM +	7	sec F	GM =	15	sec	
		T	F _(P)	4		min c	rossing	time =	7	sec	GM +	6	sec F	-GM =	13	sec	
			G _(P)	4		min c	rossing	time =	8	sec	GM +	7	sec F	-GM =	15	sec	
		— i	H _(P)	4		min c	rossing	time =	9	sec	GM +	5	sec F	-GM =	14	sec	
							10005		<u> </u>		0.01	, č			<u> </u>	000	
			<u> </u>		<u> </u>				<u> </u>						1		
			<u> </u>		+	+			<u> </u>						+	+	
			├───	<u> </u>		┼──			├──	<u> </u>					┼──	┼──	
			<u> </u>	<u> </u>	<u> </u>				<u> </u>	<u> </u>		<u> </u>			<u> </u>		
AM Traffic Flow (pcu/hr	1)		N	PM Traffic F	Flow (pcu/hr)	(N	S = 1940	+ 100 (W-3	.25) S =	= 2080 + 10	JO (W-3.25)	Note:		
	1	I		1			1			SM = S / ((1 + 1 <u>.5 f/r)</u>	SM =	(S - 2 <u>30) / (</u>	(1 + 1 <u>.5 f/r)</u>	,		
	59 🛶	91		1		25	• -	48			AM	Peak	PM	Peak	1		
│→	▶ 943		I	1	 →	583			I	1	1+2+3		1+2+3				
			I	1					I	Sumv	0 449		0.356				
		609 🗕					756	; 🛶 👝		Sum y	27		27	<u> </u>	1		
	5 + 30	Ļ	I	1	2	, • _ •	6	ţ	I	L (S)	120		400	├───			
		35	I	1			C	18	I	C (s)	120		120	├──	ł		
			I	1					I	practical y	0.698	──	0.698	──	4		
<u> </u>				L						R.C. (%)	55%		96%	<u> </u>	L		
1	<u>.</u>	2				3				4				5			
							D1 D2				+	H _(P)	I				
										1	i Em	Green	t 				
62	←A1		←			◆	┵ᆃ└ୢ			1	(P)	-64	:				
	▲ A2		 B1							1	ν 4	- → `	′ I				
			D .										I				
						L				L				L			
AM G =	-	I/G = 5	G =		I/G =	5	G =		I/G =	2	G =	15	I/G =	3	G =		
G =	=	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		
PM G =	-	I/G = 5	G =		I/G =	5	G =		I/G =	2	G =	15	I/G =	3	G =		
G =	=	I/G =	G =		I/G =		G =		I/G =		G =		I/G =		G =		

Junction:	Hona Po Ro	oad / San Hi	ng Roac												Job Nu	mber:	J7265
Scenario:	With Approv	ved Scheme	<u>ig nes.</u>											•	002	P.	47
Design Year:	2033	Designe	ed By:					Checke	ed By:					Date:	2	1 <u>Feb 20</u>)24
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill	Turning %	Sat. Flow	AM Peak Flow	y value	Critical y	Turning %	Sat. Flow	PM Peak Flow	y value	Critical y
Hong Po Road W	/B	SA+LT	A1	1	3.75	10.0	Giduion	11	1958	308	0.157		5	1975	372	0.188	
		SA	A2	1	3.75				2130	336	0.158		-	2130	402	0.189	0.189
Access Road NB	j.	LT+RT	B1	2	3.50	15.0		100	1786	35	0.020		100	1786	8	0.004	
Hong Po Road El	В	LT	C1	1	3.65	25.0	ļ'	100	1868	22	0.012		100	1868	35	0.019	
		SA	C2	1	3.65	 	ļ'		2120	472	0.223	0.223		2120	292	0.138	
		SA	C3	1	3.65		'	 '	2120	471	0.222	!		2120	291	0.137	\mid
Cur Lling Dood C				—	──	──	'	 '	──			!	┟───┤			'	
San Hing Koau S	;B		D1	3	3.65	10.0	'	100	1722	80	0.046	0.046	100	1722	37	0.021	0.021
		LITIN	D2	3	3.65	15.0	'	100	1800	84	0.047		100	1800	39	0.022	
			[_]	├──	<u> </u>	├──		╞──┘	├──				┟──┤			<u> </u> '	├
			I	├				'	├───			+	┟──┤				<u> </u>
			!	<u> </u>													<u> </u>
			i														
			i														
						İ											
		_							Ĺ								
pedestrian pha	ise		E _(P)	4	<u> </u>	min c	rossing	time =	8	sec	GM +	7	sec F	GM =	15	sec	<u> </u>
			F _(P)	4	 	min c	rossing	time =	7	sec	GM +	6	sec F	GM =	13	sec	
			G _(P)	4		min c	rossing	time =	8	sec	GM +	7	sec F	GM =	15	sec	
			H _(P)	4	──	min c	rossing	time =	11	sec	GM +	9	sec F	GM =	20	sec	
			I _(P)	2,3,4	──	min c	rossing	time =	5	sec	GM +	9	sec F	GM =	14	sec	──
			[]	├──	├──	<u> </u>			┣───								
			[_]	├──	<u> </u>	├──			├──			<u> </u>				<u> </u> '	├
• M Tffic Flow (pou/br				DM Traffic		<u> </u>			<u> </u>				·		Int-to:		<u> </u>
AM Traffic How (pcumi)		N	PM Tramer	·low (pcu/ni)				N	S = 1940 ·	+ 100 (W-3	.25) S =	2080 + 10	0 (W-3.25)	Note:		
										SM = S / (1 + 1.5 f/r)	SM =	(S - 230) / (1 + 1.5 f/r)	4		
	2 65 ← ·	→ 99	' I		30	20	← └→	50	'		AM	Peak	PM I	Peak	-		
	943		ļ			583			I		1+3	<u> </u>	1+3		4		
		609 🗕	. !				756	•		Sum y	0.269		0.210		1		
(5 < _ > 30	25	ļ		2		6	↓ ↓	ļ	L (s)	120		41				
		35	ļ				-	18	ļ	C (S)	0.593		0 593		1		
			ļ						ļ	R.C. (%)	120%		182%				
						1				1				-			,
1 ▲		2				3		24 02		4	(P)	H _(P)	ļ	5			
C1 C2	•										t_` ←		t				
C3	←A1		← →				+	┶┶			E(P)	G(P)					
	A2		 B1							1	← – F _(P)		*				
AM G=			G =	5	I/G =	5	G =		I/G =	10	G =	15	I/G =	3	G =		
G =	=	/G = 5	G =	0	1/G =	5	- G =		" - I/G =	10	- G =	15	1/G =	3	- G =		
PM G =	-	I/G = 5	G =	5	I/G =	5	G =		I/G =	10	G =	15	I/G =	3	G =		
G =	-	I/G = 5	G =		I/G =	5	G =		I/G =	10	G =	15	I/G =	3	G =		

Junction:	Hong Po Ro	ad / San Hi	ing Road	t											Job Nu	mber:	J7265
Scenario:	With Propos	ed Scheme	<u>}</u>													Ρ.	48
Design Year:	2033	Designe	ed By:				•	Checke	d By:					Date:	21	Feb 20	24
										AM Peak					PM Peak		
	Approach		Phase	Stage	Width (m)	Radius (m)	% Up-hill Gradient	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y	Turning %	Sat. Flow (pcu/hr)	Flow (pcu/hr)	y value	Critical y
Hong Po Road W	VB	SA+LT	A1	1	3.75	10.0		11	1958	308	0.157		5	1975	372	0.188	
		SA	A2	1	3.75	<u> </u>			2130	336	0.158			2130	402	0.189	0.189
					<u> </u>	<u> </u>									L	<u> </u>	
Access Road NB	3	LT+RT	B1	2	3.50	15.0		100	1786	35	0.020		100	1786	8	0.004	
					┝──	<u> </u>									<u> </u>		
Hong Po Road E	B		C1	1	3.65	25.0		100	1868	24	0.013		100	1868	37	0.020	
		SA	C2	1	3.65	<u> </u>			2120	472	0.223	0.223		2120	292	0.138	
		54	C3	1	3.65	<u> </u>			2120	471	0.222			2120	291	0.137	
San Hing Road S	2B			-					1700					1=00			
San Hing Road S	90		D1	3	3.65	10.0		100	1722	92	0.053	0.053	100	1722	40	0.023	0.023
		Linti	D2	3	3.65	15.0		100	1800	96	0.053		100	1800	41	0.023	
					<u> </u>												
					<u> </u>	<u> </u>										<u> </u>	
pedestrian pha	ise		E _(P)	4		min c	rossing	time =	8	sec	GM +	7	sec F	GM =	15	sec	
	130		–(F) F _(P)	4	<u> </u>	min c	rossing	time =	7	sec	GM +	6	sec F	GM =	13	sec	
			G _(P)	4		min c	rossing	time =	, 8	sec	GM +	7	sec F	GM =	15	sec	
			H _(P)	4		min c	rossing	time =	11	sec	GM +	9	sec F	GM =	20	sec	
			I _(P)	2.3.4	†	min c	rossing	time =	5	sec	GM +	9	sec F	GM =	14	sec	
				_,c, .			1000				0		000	<u>c</u>			
					†	†											
					†	†											
AM Traffic Flow (pcu/h	n		NI	PM Traffic	Flow (pcu/hr)										Note:		
	•)		N						N	S = 1940 ·	+ 100 (W-3	.25) S =	2080 + 100) (W-3.25)			
2				1	27	20	Ι.	-0		SM = S / (1 + 1.5 f/r)	SM = (S - 230) / (1 + 1.5 f/r)	l		
	4 /4 🖛 -	→ 114	'	1	`	20	د ' ب	53			AM	Peak	PM	Peak	İ		
,	943					583					1+3		1+3				
		600 4					750			Sum y	0.276		0.212				
	5 4 2 20	609 -			2	<u> </u>	6			L (s)	41		41				
	5 5 5 50	35			2	` ´	0	18		C (s)	120		120				
										practical y	0.593		0.593				
										R.C. (%)	115%		180%				
1		2				3				4	I _(P)			5			
C1								D1 D2			`× +	H _(P)					
	•						•	┶└			E _(P)	G _(P)					
	A1 A2		$\overset{\bullet}{\top}$						l		•	-• (,				
	¥		B1								F _(P)						
AM G =	= L	/G = 5	G =	5	I/G =	5	G =		I/G =	10	G =	15	I/G =	3	G =		
G =	: l	G= 5	G =		I/G =	5	G =		I/G =	10	G =	15	I/G =	3	G =		
PM G =	: b	G= 5	G =	5	I/G =	5	G =		I/G =	10	G =	15	I/G =	3	G =		
G =	= L	/G = 5	G =		l/G =	5	G =		I/G =	10	G =	15	I/G =	3	G =		

Appendix B – Public Transport Survey Result

TABLE B1DETAILED INFORMATION OCCUPANCY SURVEY RESULT ON THE
PUBLIC TRANSPORT NEAR THE SUBJECT SITE

Direction	Routes	АМ				РМ			
		No. of	f No. of Passenger Occu-			No. of	No. of Passenger Occu-		
		Trips	Capacity	Occupied	pancy	Trips	Capacity	Occupied	pancy
		-	[a]	[b]	[c]=[b]/[a]	-	[a]	[b]	[c]=[b]/[a]
To other	CTB 50	3	405	203	50%	0	-	-	-
districts	CTB 55	3	405	270	67%	0	-	-	-
	CTB 56	0	-	-	-	0	-	-	-
	CTB 56A	4	540	378	70%	0	-	-	-
	CTB 950	2	270	169	63%	0	-	-	-
	CTB 955	1	135	68	50%	0	-	-	-
	СТВ ВЗА	1	135	122	90%	1	135	68	50%
	KMB 53	2	270	102	38%	2	270	135	50%
	KMB 63X	5	675	380	56%	2	270	68	25%
	KMB 67M	12	1620	354	22%	5	675	68	10%
	KMB 67X	10	1350	280	21%	3	405	102	25%
	KMB 68A	6	810	439	54%	3	405	152	38%
	KMB 258P	2	270	135	50%	0	_	_	-
	KMB 261P	5	675	169	25%	0	-	-	-
	KMB 267X	3	405	122	30%	0	-	-	-
	KMB 960A	2	270	27	10%	0	-	-	-
	KMB 960C	2	270	14	5%	0	-	_	-
	KMB 960P	6	810	463	57%	0	-	_	-
	KMB 960X	9	1215	152	13%	0	-	_	-
	LWB A34	2	270	50	18%	1	135	11	8%
	LWB E33P	4	540	115	21%	0	_	_	-
	NLB B2	3	405	203	50%	3	405	152	38%
	GMB42	4	76	46	60%	4	76	31	40%
From other	CTB 50	0	_	_	-	3	405	102	25%
districts	CTB 55	0	-	_	-	3	405	102	25%
	CTB 56	0	-	_	-	0	-	-	-
	CTB 56A	2	270	135	50%	3	405	304	75%
	CTB 950	0	_	_	_	1	135	68	50%
	CTB 955	0	-	_	-	1	135	81	60%
	СТВ ВЗА	1	135	68	50%	2	270	203	75%
	KMB 53	2	270	102	38%	2	270	203	75%
	KMB 63X	2	270	102	38%	3	405	304	75%
	KMB 67M	6	810	61	8%	10	1350	422	31%
	KMB 67X	4	540	75	14%	7	945	237	25%
	KMB 68A	3	405	237	58%	5	675	394	58%
	KMB 258P	0	-	-	-	3	405	102	25%
	KMB 261P	0	-	_	-	1	135	54	40%
	KMB 267X	0	-	_	-	2	270	68	25%
	KMB 960A	0	-	_	-	1	135	54	40%
	KMB 960C	0	-	_	-	1	135	68	50%
	KMB 960P	0	-	_	-	2	270	68	25%
	KMB 960X	0	-	-	_	9	1215	608	50%
	LWB A34	0	-	-	_	2	270	27	10%
	LWB E33P	0	-	-	_	3	405	41	10%
	NLB B2	3	405	178	44%	3	405	254	63%
	GMB42	4	76	20	26%	4	76	25	33%

Appendix C – Planned Road Works to be implemented by the Owner Proposed Rezoning from "Residential (Group B)1" Zone to "Residential (Group B)4" Zone for Medium-Density Housing Development to Include a Footpath for Public use at Various Lots and Adjacent Government Land in DD130, Lam Tei, Tuen Mun (Application no. Y/TM-LTYY/11)



Appendix D – Swept Path Analysis








Appendix E – Extract of Planned Road Works under Agreement No. CE 39/2021 (CE) by CEDD





S:*CV201101*Tsing Lun Road and Tsz Tin Road Junction*Sketches*LWSK-J5-0009.dgn
19/09/2022 **10.9.25.31*CAD_General*CAD_Common*FEN_TABLE*filename&date.tbl

	NOTES :	
POSED SITE NDARY	 ALL DIMENSIONS ARE IN MILLIMETRES UNLESS O STATED. 	N THERWISE
STING SINGLE POST H TRAFFIC SIGN OR	2. GRID LINES ARE IN HO METRIC GRID 1980.	NG KONG
EET NAME PLATE TO RELOCATED	3. ALL LEVELS ARE IN ME PRINCIPAL DATUM (mPD	TRES ABOVE).
POSED SYMBOLIC FFIC SIGN NO. ING INDICATED ECTION	 THIS DRAWING SHALL BE CONJUNCTION WITH DRAW LWSK-J5-0010. 	E READ IN WING NO.
POSED ROAD MARKING E NO.	5. DETAILS OF CONCRETE F TRAFFIC BOLLARD SHALL Hyd STANDARD DRAWING H2140 TO H2141.	PLINTH FOR REFER TO NOS:
STING ROAD MARKING BE REMOVED	6. FOR DETAILS OF TRAFF AND ROAD MARKING, REF VOLUME 3 OF TRANSPOR AND DESIGN MANUAL (TF PUBLISHED BY TRANSPOR DEPARTMENT.	ICSIGNS FERTO FPLANNING PDM) RT
•	 FOR SUPPORT DETAILS (POST TRAFFIC SIGNS IN COLOR OF POST, REFER STANDARD DRAWING NOS H2148. 	DF SINGLE NCLUDING TO HyD H2147 TO
	8. BACK OF ALL SIGNS SH PAINTED IN GREY TO BS CODE 18B19.	ALL BE 55252F
	9. EXISTING ROAD MARKING AFFECTED SHALL BE REN REPLACED BY PROPOSED MARKING.	G TO BE MOVED AND ROAD
	10. NEW SUPPORTS, INCLUD SINGLE/MULTIPLE POSTS FRAME SUPPORTS, SHALL CONSTRUCTED FOR ALL F AND RELOCATED TRAFFIC DIRECTIONAL SIGNS,	ING 5 AND - BE PROPOSED C OR
	works order title IMPROVEMENT WORKS AT JUNCTION OF TSING LUN ROAD AND TSZ TIN ROAD, TUEN MUN	
	drawing title TRAFFIC AIDS AND ROAD MARKING LAYOUT	
	drawing no.	scale
SCALE BAF	LWSK-J5-0009	1 : 250
	office LAND WORKS DIVISION CIVIL ENGINEERING OFFICE	
250 SCALE BAR PYRIGHT RESERVED	CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT	

A3 420 x 297

Appendix F – Extract of Planned Road Works under Agreement No. CE 01/2020 (CE) by CEDD

