

Appendix 7

Sewerage Impact Analysis

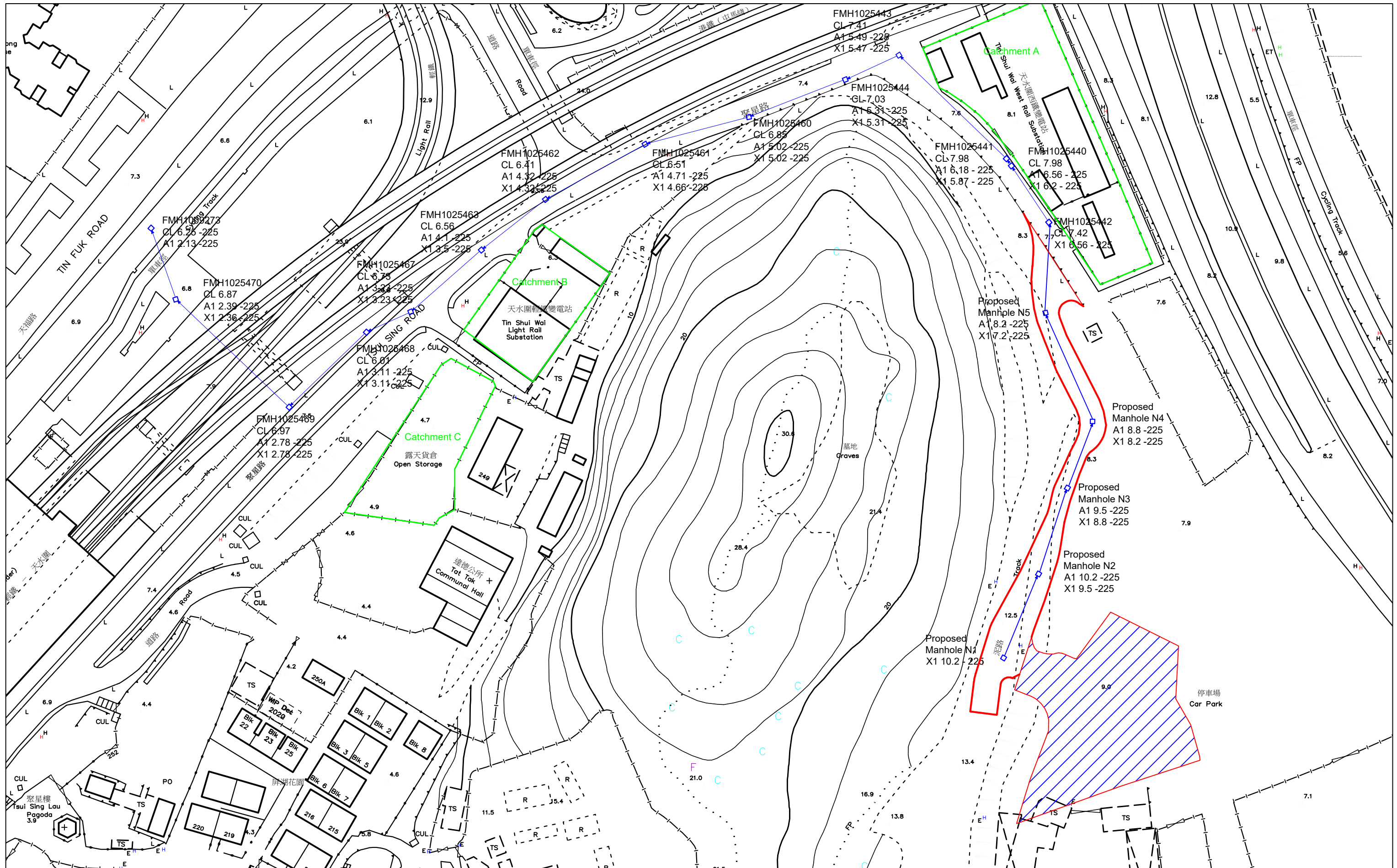


Table 1 Calculation for Sewage Generation Rate of the Proposed Development

RCHE Use		
Total Number of Residents	420	
Unit Flow Factor	0.19	GESF(Table T-1) - UFF for Domestic Flows 0.190 (Institutional and special class)
Total Number of Employee	150	
Unit Flow Factor	0.28	GESF(Table T-2) - UFF for Commercial Flow and Student Flow 0.08 (Commerical Employee) + 0.2 (Community, Social & Personal Services)
Estimated Dry Weather Flow	121.80	m ³ /day
Senior Hostel		
Estimated Population	18	
Unit Flow Factor	0.27	GESF(Table T-1) - UFF for Domestic Flows 0.270 (Private R2)
Estimated Dry Weather Flow	4.86	m ³ /day
Total Flow from Proposed Development		
Flow Rate	126.66	m ³ /day
Catchment Inflow Factor	1.00	Catchment Inflow Factor = 1.00 (Yuen Long) based on EPD's GESF Table T-4
Flow Rate (including catchment inflow Contributing Population)	126.66 469.11	m ³ /day
Peaking Factor	8	Peaking Factor=8 for population (including stormwater allowance) base on EPD's GESF Table T-5
Peak Flow	11.73	L/s

Table 2 Calculation for Sewage Generation

		Remarks
A	1a Tin Shui Wai West Rail Substation	
	Assumed number of employees	5
	Unit Flow Factor	0.33 m ³ /person/day
	Estimated Dry Weather Flow	1.65 m ³ /person/day
B	2 LRT Rectifier Station	
	Assumed number of employees	5
	Unit Flow Factor	0.33 m ³ /person/day
	Estimated Dry Weather Flow	1.65 m ³ /day
C	3 North Site of the approved Transitional Housing	
	Estimated number of residents	114
	Unit Flow Factor	0.27
	Estimated Dry Weather Flow	30.78 m ³ /day

Table 3 Total Estimated Peak Flow

Catchment	From the Most Upstream	Total Estimated Dry Weather Flow (m ³ /day)	Catchment Inflow Factor ^[1]	Cumulative Average Dry Weather Flow (m ³ /day)	Contributing Population ^[2]	Peaking Factor ^[3]	Total Estimated Peak Flow (m ³ /day)	Total Estimated Peak Flow (L/s)
	Proposed Development	126.66	1	126.66	469.11	8	1013.28	11.73
A	Tin Shui Wai West Rail Substation	1.65	1	128.31	475.22	8	1026.48	11.88
A to B	LRT Rectifier Station	1.65	1	129.96	481.33	8	1039.68	12.03
A to C	North Site of the approved Transitional Housing	30.78	1	160.74	595.33	8	1285.92	14.88

Remarks:

^[1] Catchment Inflow Factor = 1.00 (Yuen Long) based on EPD's GESF Table T-4

^[2] Based on the equation from GESF: $Contributing\ Population = \frac{Calculated\ total\ average\ flow\ (m^3/day)}{0.27\ (m^3/person/day)}$

^[3] Peaking Factor=8 for population <1000, and 6 for population 1000-5000 (including stormwater allowance) base on EPD's GESF Table T-5

Table 4 Sewer Capacity Check

Catchment	Pipe Name	Manhole Reference		Pipe Diameter (m)	Pipe Length (m)	Invert Level (mPD)		g (m/s ²)	k _s (m)	s	v (m ² /s)	V (m/s)	Area (m ²)	Q (m ³ /s)	Sewer Capacity (L/s)	Sewage Flow (L/s)	% of Peak Flow to Sewer Capacity
		Upstream	Downstream			Upstream	Downstream										
Proposed Development	S1	N1	N2	0.225	25	10.2	9.5	9.81	0.003	0.0280	0.00000114	1.7149	0.040	0.068	68.19	11.73	17.20%
	S2	N2	N3	0.225	25	9.5	8.8	9.81	0.003	0.0280	0.00000114	1.7149	0.040	0.068	68.19	11.73	17.20%
	S3	N3	N4	0.225	19.4	8.8	8.2	9.81	0.0006	0.0309	0.00000114	2.3071	0.040	0.092	91.73	11.73	12.78%
	S4	N4	N5	0.225	33	8.2	7.2	9.81	0.0006	0.0303	0.00000114	2.2836	0.040	0.091	90.80	11.73	12.92%
	S5	N5	FMH1025442	0.225	25	7.2	6.56	9.81	0.0006	0.0256	0.00000114	2.0977	0.040	0.083	83.41	11.73	14.06%
A	FWD1028401	FMH1025442	FMH1025440	0.225	18.2	6.56	6.2	9.81	0.0006	0.0198	0.00000114	1.8422	0.040	0.073	73.25	11.88	16.22%
	FWD1028402	FMH1025440	FMH1025441	0.225	1.2	6.2	6.18	9.81	0.003	0.0167	0.00000114	1.3224	0.040	0.053	52.58	11.88	22.60%
	FWD1028403	FMH1025441	FMH1025443	0.225	41.8	5.87	5.49	9.81	0.003	0.0091	0.00000114	0.9759	0.040	0.039	38.80	11.88	30.62%
	FWD1028404	FMH1025443	FMH1025444	0.225	15.8	5.47	5.31	9.81	0.003	0.01013	0.00000114	1.0301	0.040	0.041	40.96	11.88	29.01%
	FWD1028420	FMH1025444	FMH1025460	0.225	28.5	5.31	5.02	9.81	0.003	0.01018	0.00000114	1.0326	0.040	0.041	41.06	11.88	28.94%
	FWD1028421	FMH1025460	FMH1025461	0.225	29.7	5.02	4.71	9.81	0.003	0.01044	0.00000114	1.0459	0.040	0.042	41.59	11.88	28.57%
	FWD1028422	FMH1025461	FMH1025462	0.225	31.5	4.66	4.32	9.81	0.003	0.01079	0.00000114	1.0636	0.040	0.042	42.29	11.88	28.09%
	FWD1028424	FMH1025462	FMH1025463	0.225	22.4	4.32	4.1	9.81	0.003	0.00982	0.00000114	1.0144	0.040	0.040	40.34	11.88	29.45%
A+B	FWD1028427	FMH1025463	FMH1025467	0.225	25.5	3.5	3.23	9.81	0.003	0.011	0.00000114	1.0534	0.040	0.042	41.88	12.03	28.73%
A+B+C	FWD1028428	FMH1025467	FMH1025468	0.225	12.7	3.23	3.11	9.81	0.003	0.009	0.00000114	0.9950	0.040	0.040	39.56	14.88	37.62%
	FWD1028429	FMH1025468	FMH1025469	0.225	30.3	3.11	2.78	9.81	0.003	0.011	0.00000114	1.0684	0.040	0.042	42.48	14.88	35.04%
	FWD1028430	FMH1025469	FMH1025470	0.225	43.6	2.78	2.39	9.81	0.003	0.009	0.00000114	0.9680	0.040	0.038	38.49	14.88	38.67%
	FWD1028432	FMH1025470	FMH1009273	0.225	20	2.36	2.13	9.81	0.003	0.0115	0.00000114	1.0980	0.040	0.044	43.66	14.88	34.09%

Remarks:

(1) g=gravitational acceleration; k_s=equivalent pipeline roughness; s=hydraulic gradient; v=kinematic viscosity of fluid; V=mean velocity

(2) The value of k_s = 0.6mm (for velocities greater than 1.2m/s, otherwise 3mm) is adopted for the calculation of slimed clayware sewer, poor condition (based on Table 5: Recommended Roughness Values in Sewerage Manual)

(3) The mean velocity is calculated using the Colebrook-White Equation:

$$v = \frac{1}{\sqrt{(8gDs) \log\left(\frac{k_s}{3.7D} + \frac{2.51v}{D\sqrt{(2gDs)}}\right)}}$$