Annex 3

Revised Drainage Impact Assessment



Proposed Residential Development at Lot 182 S.B. in DD128, Lau Fu Shan

Drainage Impact Assessment Report

Reference: P058/01 Issue 3

Date: January 23

Confidential





Proposed Residential Development at Lot 182 S.B. in DD128, Lau Fu Shan

Drainage Impact Assessment Report

Checked and Approved by:

Patrick Ip Director

Reference: P058 Issue 3

Date: January 23

Issu e	Status	Prepared By	Date	Checked by	Date	Approved By	Date
1	For Comment	Cheryl Chan	Jul 22	Emily Tang	Jul 22	Patrick Ip	Jul 22
2	For Comment	Cheryl Chan	Dec 22	Emily Tang	Dec 22	Patrick Ip	Dec 22
3	For Comment	Cheryl Chan	Jan 23	Emily Tang	Jan 23	Patrick Ip	Jan 23

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1 Introduction

1.1 Introduction

The Applicant intends to develop a Villa at Lot 182 S.B in D.D.128 in Lau Fu Shan, New Territories (hereafter as "the Site").

According to the Approved Ha Tsuen Fringe Outline Zoning Plan (No. S/YL-HTF/12) published by Town Planning Board in October 2018, the site is currently zoned as "Residential (Group D)".

Owing to concerns on possible drainage impact arising from the proposed development. Urban Green Consultants Ltd. (UGC) has been commissioned to conduct a Drainage Impact Assessment (DIA) to demonstrate the acceptability of drainage impact upon the surrounding environment.

1.2 Study Objectives

The objectives of this DIA are to assess the possible drainage impacts may be caused by the proposed development and to recommend the mitigation measures to alleviate such impacts if necessary.

1.3 Report Structure

The remaining chapters of this report are shown below:

Chapter 2 - Site Context

Chapter 3 - Drainage Analysis

Chapter 4 - Conclusions

2 Site Context

2.1 Introduction

The Project Site is located at Lot 182 S.B in D.D.128, Lau Fu Shan. The Site falls within an area zoned "Residential (Group D)". The Site area is approximately 2,550 m².

2.2 Site Characteristics and Proposed Uses

The Site is located in Lau Fu Shan. To the east of the site is an open storage yard for recycling materials and a warehouse. To the south are open storage yards for metals and a recyclable collection centre. To the west is an open storage yard for construction materials. To the north are vacant land, shrubland and a recyclables collection centre. The Site area is approximately 2,550 m².

Figure 2.1 shows the Site Location and the environment.

2.3 Existing Drainage Conditions

Site survey was conducted on 3 December 2021 to collect the updated information of the drainage characteristics, catchments, topography, existing drainage facilities, flow path and surface type within the Site and its surrounding.

Based on the site survey and review of drainage plans (reference no.: 6-NW-1B) from Drainage Services Department (DSD) in December 2021, it has revealed that the Site is not currently served by any form of DSD's drainage facility. However, the surface runoff from the Site may be possible to discharge into an existing underground U-channel on the south-east of the Site. According to the calculation of flow capacity, the proposed discharge point and the proposed U-channels are able to catch all the runoff from the Site and identified catchments.

3 Drainage Analysis

3.1 Assessment Methodology and Assumption

This DIA has adopted the Rational Method for runoff estimation:

```
Q_p = 0.278 \ i \ \sum C_j \ A_j
where
Q_p is peak runoff (m^3/s);
i is rainfall intensity (mm/hr);
A_j is the j^{th} catchment (km^2);
C_j is the runoff coefficient of the j^{th} catchment (dimensionless).
```

The details of the Rational Method can be referred to the *Stormwater Drainage Manual* (SDM) (DSD, 2018).

Based on a 1:50 year flood protection standard in the SDM and the estimated time of concentration, the appropriate rainfall intensities (i) were calculated based on linear interpolation of the intermediate table values.

The assumptions of this DIA are summarised below:

- Rainstorm return period 1 in 50 years
- Runoff coefficient for concrete-paved area 0.95
- Runoff coefficient for flatted grassland (heavy soil) 0.25
- Runoff coefficient for steep grassland (heavy soil) 0.35
- Manning's roughness coefficient for the proposed U-channels 0.016

The existing paving condition of the Site has runoff coefficients of 0.95 for concrete, 0.25 for flat heavy soil and 0.35 for steep heavy soil, which are adopted in this DIA. It is anticipated that the extent of the existing paving condition will be improved upon approval of this application as more landscape areas have been proposed.

The capacity of the proposed U-channels has been checked by comparing with magnitudes of different combinations of the catchments. The Manning's roughness coefficient of 0.016 for U-channels (fair condition) was assumed.

3.2 Design Parameters

14 catchments (Catchments A to N) were identified based on the geographical characteristics of the Site and its nearby area as shown in Figure 3.1. As the existing U-channel is located to the south-east of the Site, the runoff from the Site will be directly discharged into the existing U-channel (E1). Figure 3.1 shows the discharge point of the Site. The surface runoff from relevant catchment has been estimated and presented in Appendix B.

Proposed peripheral U-channels (P1 and P2) will be provided along the site boundary to collect the surface runoff from catchment A-F and intercept the overland flow from catchment G-I.

As the surface runoff from catchment A-F will flow along the site boundary, the proposed underground manholes will be provided at the corner of the site boundary where walls are erected to collect the surface runoff from catchment A-F. The section of the proposed manhole is shown in Figure 3.2. The location of the proposed manholes is shown in Appendix A.

3.3 Assessment Results

Given that the Site is the undeveloped area, site modification would be made to increase concrete paving of the on-site catchment areas (i.e. Catchment G,H,I) after proposed development. The identified on-site catchment area is presented in Figure 3.1. The change in paving characteristics of the on-site catchment area is summarised in Table 3.1.

Table 3.1 Changes in Grassland and Concrete Areas

On-Site Catchment	Before Dev	velopment	After Development				
Area	Grassland	Concrete	Grassland	Concrete			
G	100%	0%	30%	70%			
н	100%	0%	30%	70%			
1	100%	0%	20%	80%			

Note: (1) The coefficient of permeable concrete is 0.3 which is reference from Permeable Interlocking Concrete Pavement. (2008). Interlocking Concrete Pavement Institute.

As summarised above, the concrete paving area for on-site catchment area is increased which imply there shall increase the surface runoff generated from the Site after proposed development. The increase of on-site catchment runoff is summarised in Table 3.2.

Table 3.2 Estimated Runoff from the Site

On-Site Catchment Area	Estimated R	Runoff, m³/s	Increase of Runoff m³/s
Alcu	Before Development	After Development	After Development
G+H+I	0.0511	0.1553	0.1042

As shown in Table 3.2, 0.1042 m³/s increased runoff will be resulted from the proposed development.

30% greenery area will be provided for Catchment G and H, while 20% greenery area will be provided for Catchment I. Estimation of the on-site runoff before and after proposed development are detailed in Table A1 and Table A2 of Appendix B.

Details calculation of the estimated proposed site catchment runoffs is shown in Table A3 of Appendix B.

Table 3.3 Estimated Proposed Site Catchment Runoffs to Existing Drainage

Channel Segment ⁽¹⁾	Diameter, m	Gradient	Capacity, m³/s	Runoff, m³/s	Catchment Served	Surfficient Capacity? (Y/N)
P1	0.375	0.05	0.451	0.307	A,B,D,E, G,H,I	Y
P2	0.375	0.05	0.451	0.106	C,F	Y
E1 ⁽³⁾	0.6	0.015	0.875	0.582	A-N, swimming pool discharge	Y

Note:

- (1) P1 and P2 are proposed U-channel. E1 is existing U-channel.
- (2) All segments (P1,P2,E1) are U-channels.
- (3) E1 is referenced from planning application of Proposed Temporary Recyclable Collection Centre for Metal and Plastic for a Period of 3 Years (Application No. A/YL-HTF/1142). The drainage plan is attached in Appendix D. If the above planning application is not approved, this project will propose segment E1.

The assessment results presented in Table 3.3 demonstrate that the proposed development has induced limited runoff to the proposed and existing U-channels.

As summarised above, the proposed development would not cause adverse drainage impacts nor increase in flooding susceptibility of the surrounding areas.

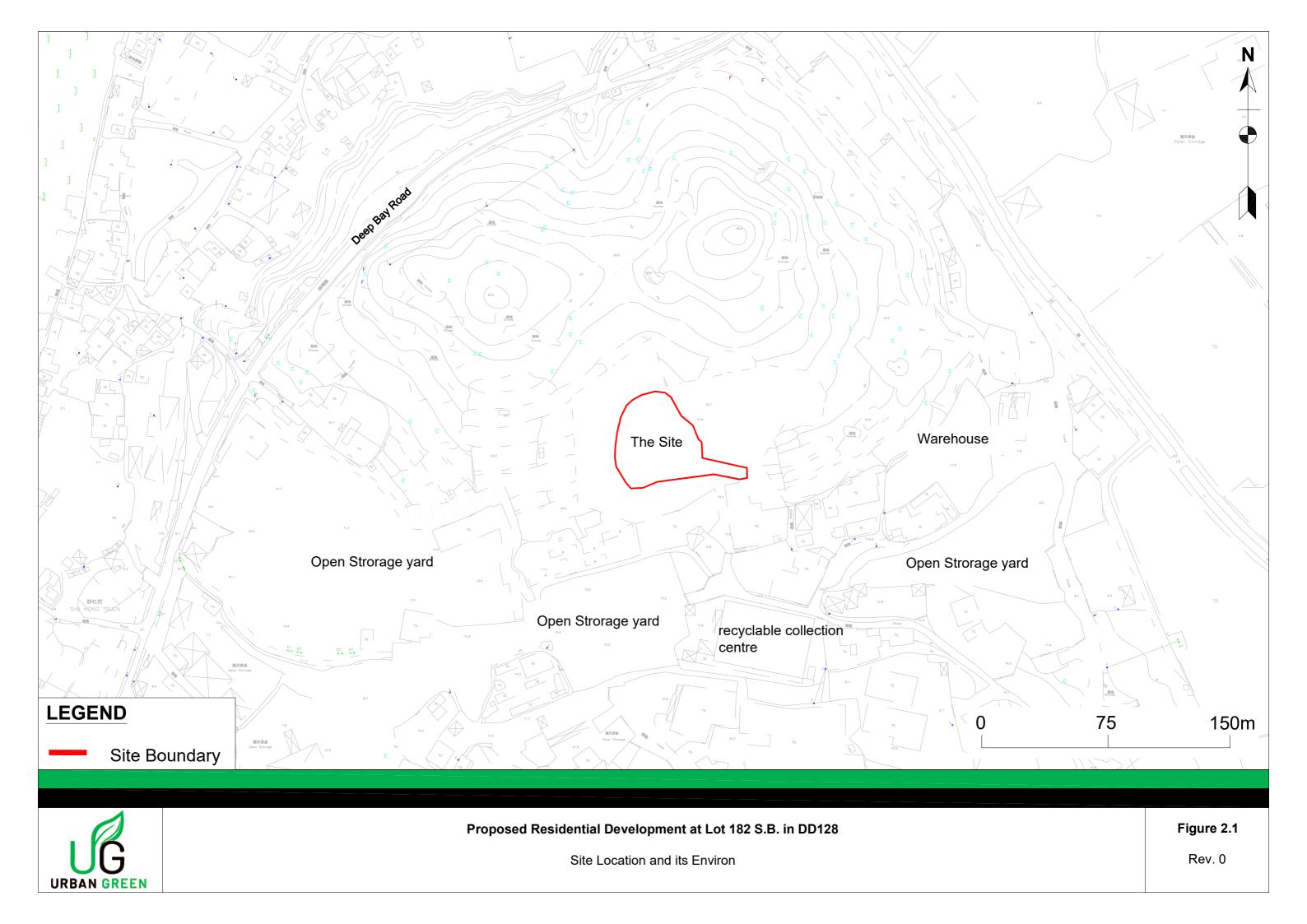
4 Conclusions

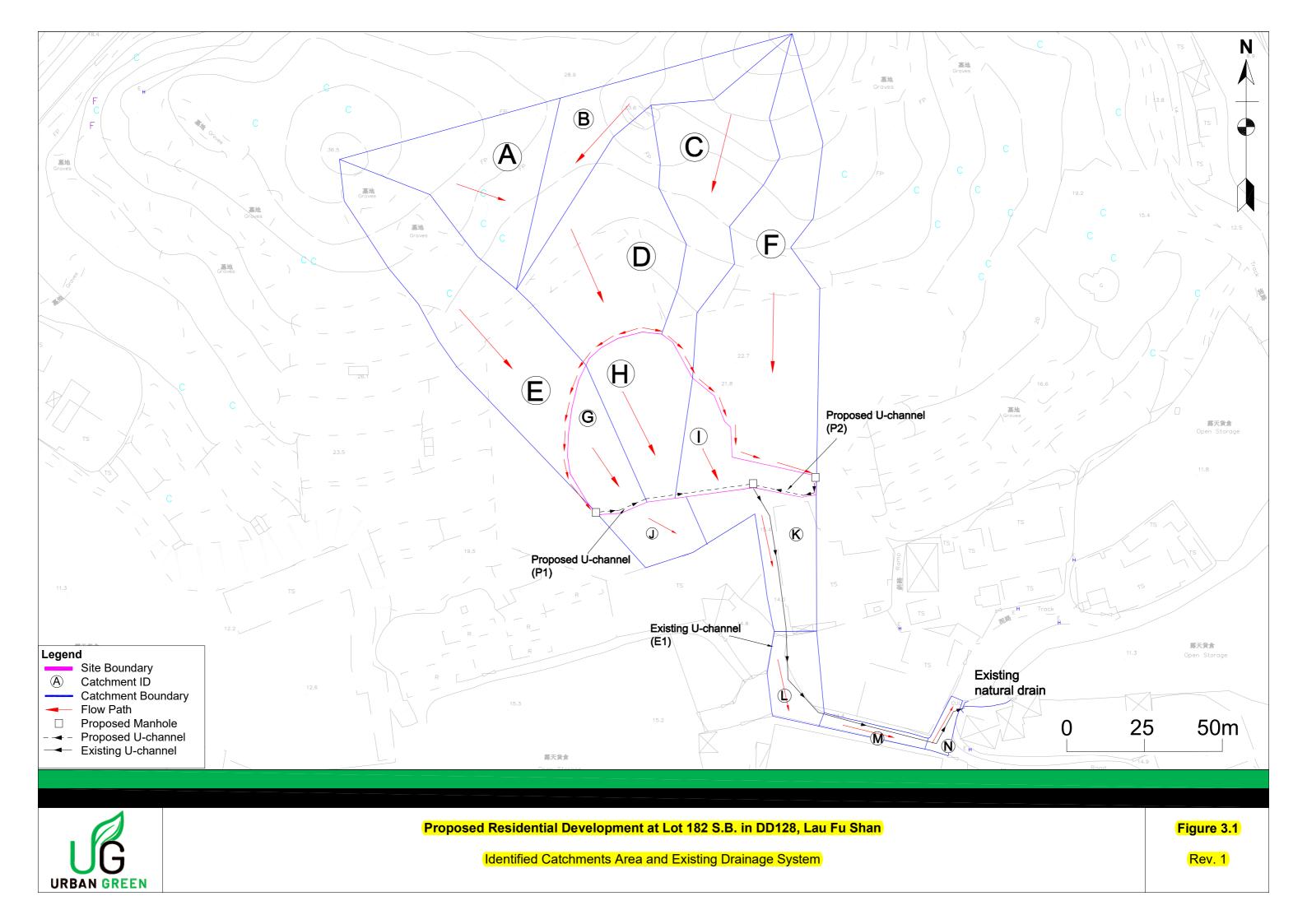
A Drainage Impact Assessment (DIA) has been conducted for the Proposed Residential Development at Lot 182 S.B. in DD128, Lau Fu Shan.

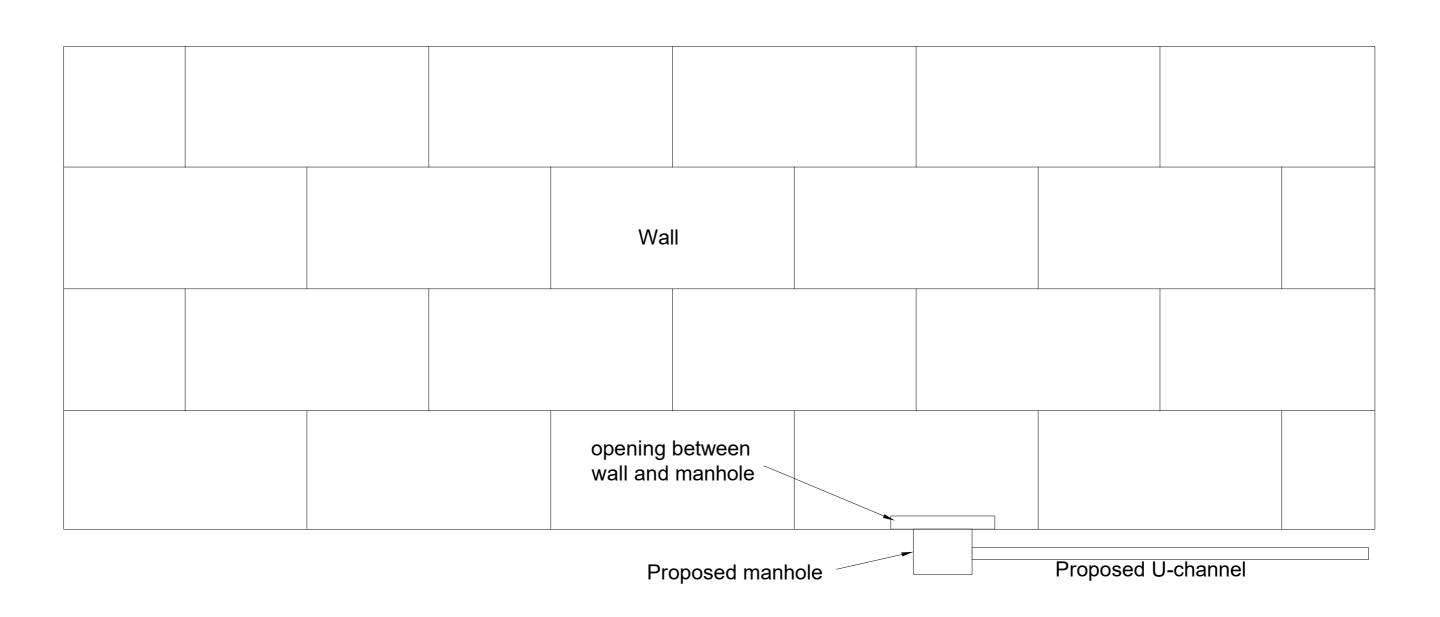
The peak surface runoff was calculated based on a 50-year return period. The assessment results have demonstrated that there shall be no adverse impact due to the proposed development. In addition, with reference to the Flooding Blackspots available on the DSD website, the Site is not located within the flooding blackspots locations/ regions. As a result, no unacceptable drainage impact is anticipated from the proposed development.

Based on the above, it is concluded that the Proposed Development shall not result in any adverse drainage impacts.

Figures



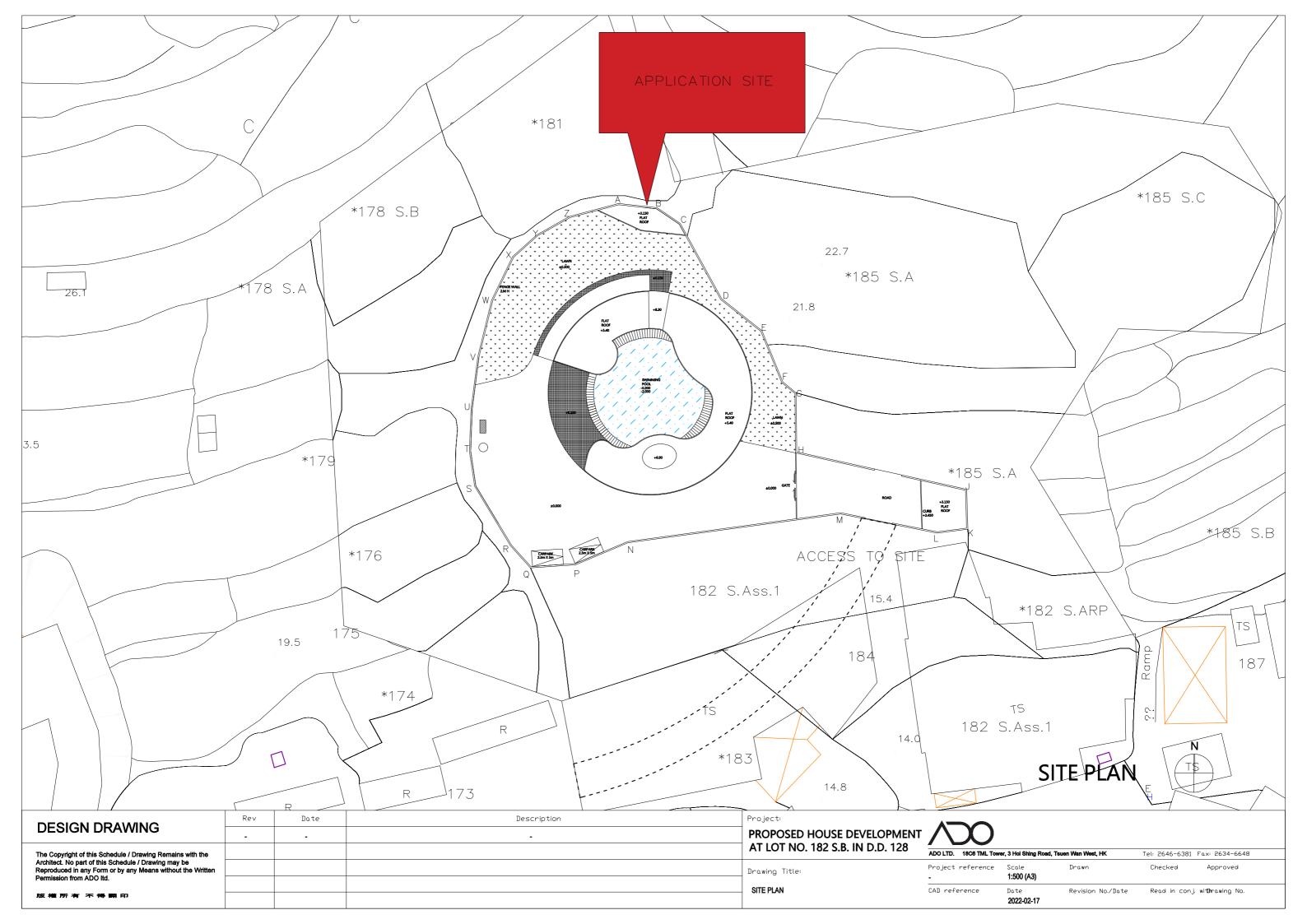


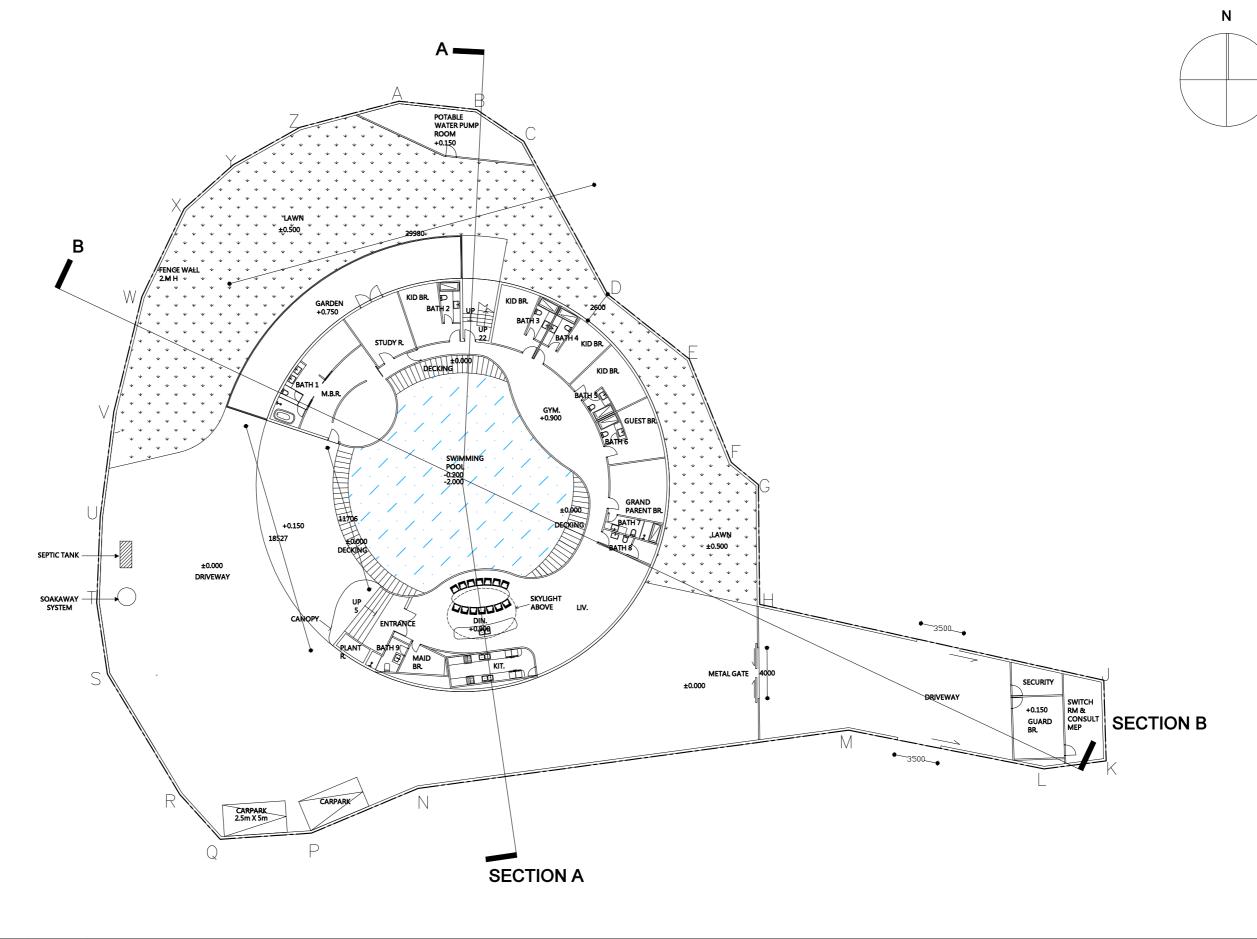




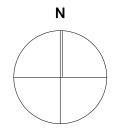
Appendix A

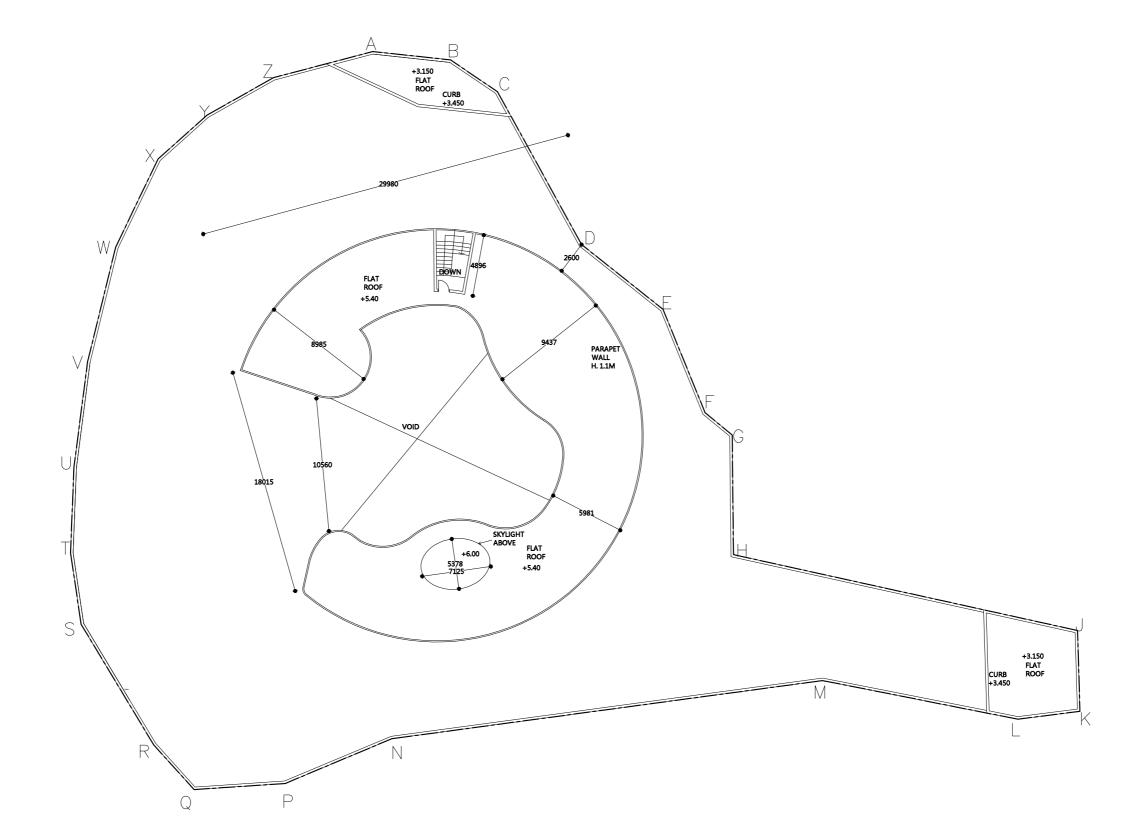
Drawings of Development Plan



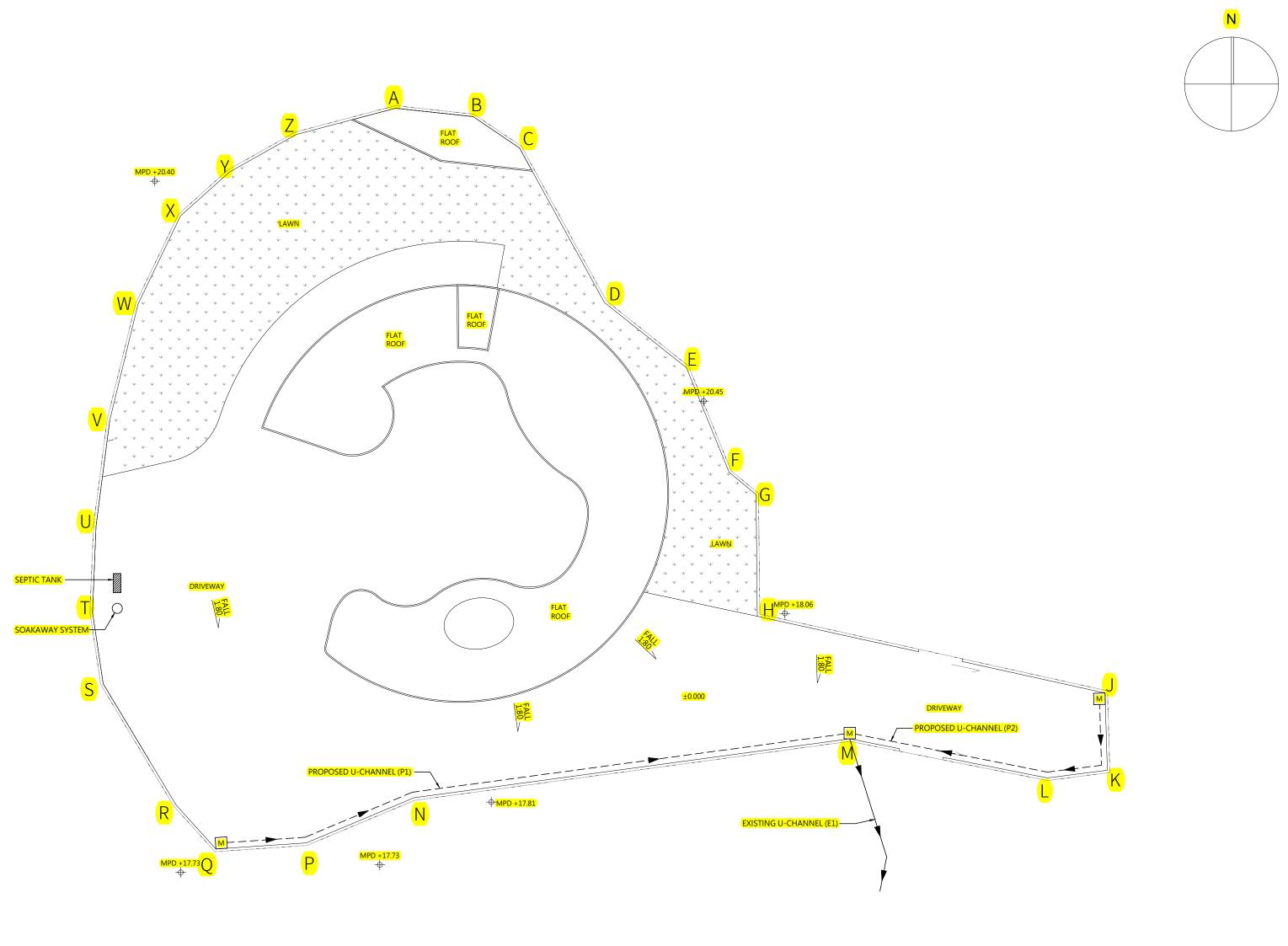


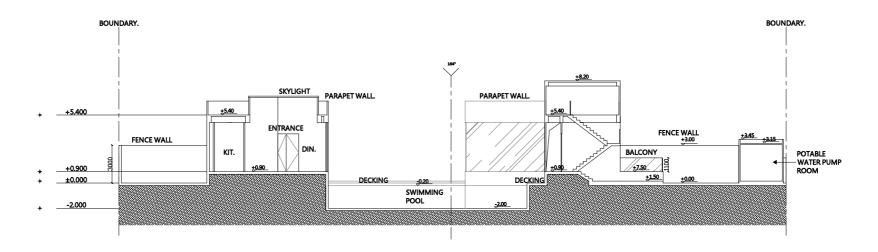
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The Copyright of this Schedule / Drawing Remains with the				AT LOT NO. 182 S.B. IN D.D. 128	ADO LTD. 18C6 TML Tow	er, 3 Hoi Shing Road,	Tsuen Wan West, HK	Tel: 2646-6381	Fax: 2634-6648
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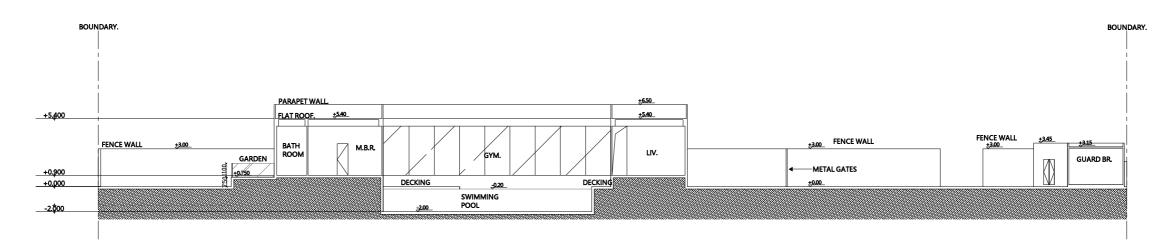


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SECTION A -A



SECTION B-B

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Appendix B

Detailed Drainage Analysis

Capacity Flows Estimation for Propose Catchments and Drainage System with 50 Year Return Period

A1.Calculation of On-Site Runoff (Existing Development)

Catchment ID	Surface Type	Catchment Area (A), m ²	Catchment Area (A), km²	Average slope (H), m/100m	Flow path length (L), m	Inlet time (t ₀), min	Time of Concentration (t _c), min	Duration (t _d), min	a (50 year return period)	b (50 year return period)	c (50 year return period)	Runoff intensity (i) mm/hr	Runoff coefficient (C)	CxA	Peak runoff (Q _p), m ³ /s
G	100% Grassland (heavysoil), flat	656	0.00066	13.79	29.00	1.30	1.30	1.30	451.3	2.5	0.34	287	0.25	0.000163948	0.0131
н	100% Grassland (heavysoil), flat	1,189	0.00119	13.89	36.00	1.52	1.52	1.52	451.3	2.5	0.34	281	0.25	0.000297337	0.0233
I.	100% Grassland (heavysoil), flat	712	0.00071	18.10	21.00	0.88	0.88	0.88	451.3	2.5	0.34	298	0.25	0.000177957	0.0148
														Total	0.0511

A2.Calculation of On-Site Runoff (After Development)

Catchment ID	Surface Type	Catchment Area (A), m ²	Catchment Area (A), km ²	Average slope (H), m/100m	Flow path length (L), m	Inlet time (t ₀), min	Time of Concentration (t _c), min	Duration (t _d), min	a (50 year return period)	b (50 year return period)	c (50 year return period)	mm/hr	Runoff coefficient (C)	CxA	Peak runoff (Q _p), m ³ /s
G	70% Concrete + 30% Grassland (heavysoil), flat	656	0.00066	13.79	29.00	1.30	1.30	1.30	451.3	2.5	0.34	287	0.74	0.000485285	0.0387
н	70% Concrete + 30% Grassland (heavysoil), flat	1,189	0.00119	13.89	36.00	1.52	1.52	1.52	451.3	2.5	0.34	281	0.74	0.000880116	0.0688
1	80% Concrete + 20% Grassland (heavysoil), flat	712	0.00071	18.10	21.00	0.88	0.88	0.88	451.3	2.5	0.34	298	0.81	0.000576581	0.0478

0.1553 Total Changes in peak runoff 0.1042

A3.Calculation of All Catchment Runoff (After Development)

Catchment ID	Surface Type	Catchment Area (A), m ²	Catchment Area (A), km²	Average slope (H), m/100m	Flow path length (L), m	Inlet time (t ₀), min	Time of Concentration (t _c), min	Duration (t _d), min	a (50 year return period)	b (50 year return period)	c (50 year return period)	Runoff intensity (i) mm/hr	Runoff coefficient (C)	CxA	Peak runoff (Q _p), m ³ /s
А	100% Grassland (heavysoil), flat	1,799	0.00180	20.73	41.00	1.53	1.53	1.53	451.3	2.5	0.34	281	0.25	0.000449755	0.0351
В	100% Grassland (heavysoil), flat	1,354	0.00135	13.49	86.00	3.59	3.59	3.59	451.3	2.5	0.34	244	0.25	0.000338476	0.0230
С	100% Grassland (heavysoil), flat	2,072	0.00207	18.35	85.00	3.20	3.20	3.20	451.3	2.5	0.34	250	0.25	0.00051792	0.0360
D	100% Grassland (heavysoil), flat	2,451	0.00245	17.67	60.00	2.24	2.24	2.24	451.3	2.5	0.34	266	0.25	0.000612733	0.0453
E	100% Grassland (heavysoil), flat	2,806	0.00281	19.07	97.00	3.52	3.52	3.52	451.3	2.5	0.34	245	0.25	0.0007015	0.0478
F	10% Concrete + 90% Grassland (heavysoil), flat	3,278	0.00328	17.88	104.00	3.76	3.76	3.76	451.3	2.5	0.34	242	0.32	0.00104896	0.0705
G	70% Concrete + 30% Grassland (heavysoil), flat	656	0.00066	13.79	29.00	1.30	1.30	1.30	451.3	2.5	0.34	287	0.74	0.000485285	0.0387
н	70% Concrete + 30% Grassland (heavysoil), flat	1,189	0.00119	13.89	36.00	1.52	1.52	1.52	451.3	2.5	0.34	281	0.74	0.000880116	0.0688
1	80% Concrete + 20% Grassland (heavysoil), flat	712	0.00071	18.10	21.00	0.88	0.88	0.88	451.3	2.5	0.34	298	0.81	0.000576581	0.0478
J	80% Concrete + 20% Grassland (heavysoil), flat	464	0.00046	30.00	10.00	0.40	0.40	0.40	451.3	2.5	0.34	314	0.81	0.00037584	0.0328
\sim	Grassland (heavysoil), flat	~ (138 ~	0.00111	~~22~~	~8.0 ~	169	~6.0		1813	~~ .5 ~ ~	~064~	── 30×		0,000,63,6	A doese A
L	100% Concrete	467	0.00047	4.17	24.00	1.41	1.41	1.41	451.3	2.5	0.34	284	0.95	0.000443249	0.0350
М	100% Concrete	144	0.00014	3.33	30.00	2.07	2.07	2.07	451.3	2.5	0.34	269	0.95	0.00013722	0.0103
N	100% Concrete	107	0.00011	10.00	14.00	0.80	0.80	0.80	451.3	2.5	0.34	301	0.95	0.00010163	0.0085
	•													Total	0.5642

Runoff is calculated in accordance with DSD's "Stormwater Drainage Manual - Planning, Design and Management" (SDM), fifth edition, January 2018...

Equation used: 0.144551

0.14465L

 $\overline{(t_d + b)^c}$

 $Q_p = 0.278 C i A$

Swimming Pool Discharge Peak flow rate, m³/s 0.0184

(1) The volume of swimming pool = annual cleaning effluent = 397m³
(2) The design cleaning time = 6 hrs
(3) The design cleaning flow rate = (1)/(2)*60*60 = 0.0184m³/s

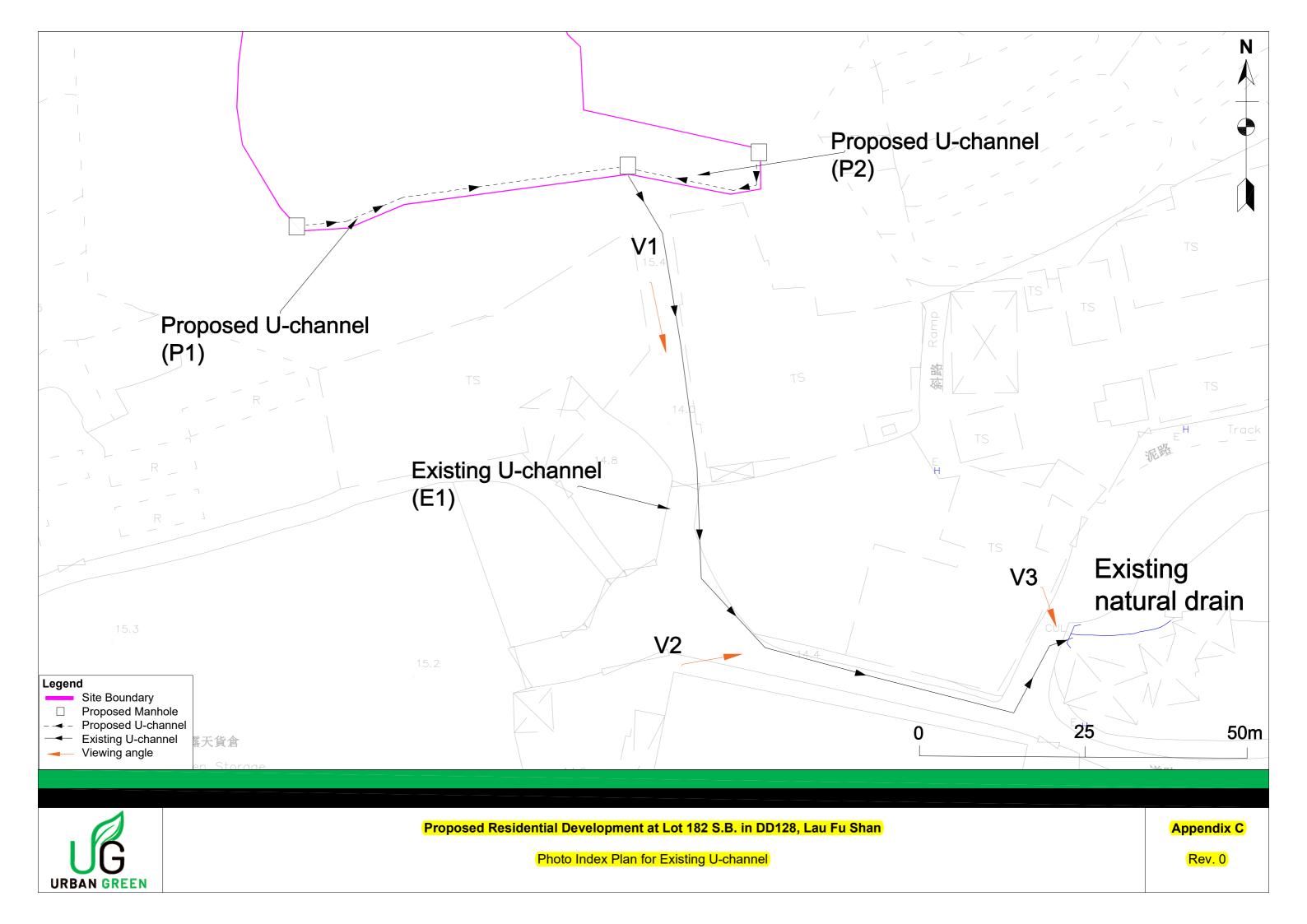
B. Contribution Es	stimation and Adequacy Checi	tor Existing Dr	ainage System	(For Ennanced Des	ign)										
Point (Channel No.)	Channel	Diameter, m	Depth, m	Slope	Length, m	Manning's Roughness Coefficient	Cross Section Area,m2	Wetted Perimeter,m	Hydraulic radius,m	Mean Velocity,m/s	Capacity flow,m3/s	Catchment Served	Runoff,m3/s	% of capacity flow	Sufficient Capacity(Y/ N)
P1	U-channel	0.375	0.375	0.050	49.0	0.016	0.126	0.964	0.130	3.590	0.451	A,B,D,E,G,H,I	0.307	68%	Y
P2	U-channel	0.375	0.375	0.050	22.4	0.016	0.126	0.964	0.130	3.590	0.451	C,F	0.106	24%	Y
F1	U-channel	0.600	0.600	0.015	125.6	0.016	0.321	1.542	0.208	2 724	0.875	A-N, swimming pool	0.582	67%	Y

Swimming pool discharge

Pool area	220 m ²	Provided by Project Architect
Pool depth	1.8 m	Provided by Project Architect
Volume	397 m ³	
Annual cleaning effluent	397 m ³	
Max. flow allowed Shortest cleaning flow time	1.12 m³/s 5.907738 min	Provided by Project Team
Design cleaning time	6 hr	Provided by Project Team
Design cleaning flow rate	0.01838 m ³ /s	

Appendix C

Photos of Existing U-channel

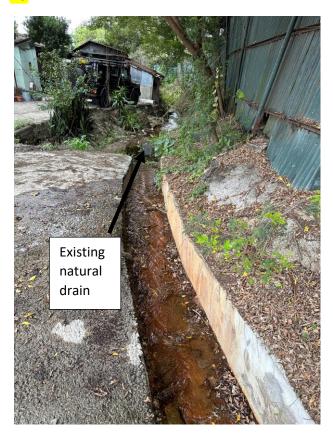


V1



V2



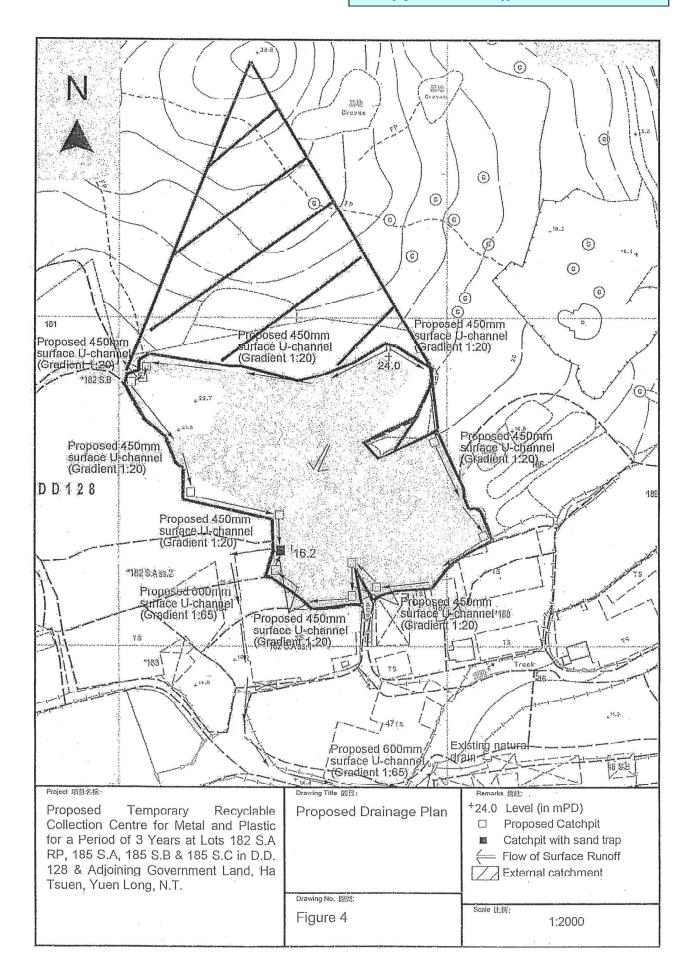




Appendix D

Reference Drainage Plan (Planning No.: A/YL-HTF/1142)

This page is extracted from applicant's submitted documents.



Application No.		A/VI-HTE/1142	
**	A/YL-HTF/1142		
Plan Area	Ha Tsuen Fringe		
District	Tuen Mun & Yuen Long West District		
Date of Application Received	19/10/2022		
Location	Lots 182 S.A RP, 185 S.A, 185 S.B and 185 S.C in D.D.128 and Adjoining Government Land, Ha Tsuen, Yuen Long, New Territories		
Proposal	Proposed Temporary Recyclable Collection Centre for Metal and Plastic for a Period of 3 Years		
Applicant's Submission	No softcopy provided by Applicant. Hardcopies available at Planning Enquiry Counters for inspection.		
Newspaper Notice & Gist of Development Proposal(including Location Plan)(indicative only for section 12A application)	Newspaper Notice 📆		
	Gist 📆		
	[Note: The Gist may be superseded by a new Gist if Further Information is received]		
No. of Comments on Application Already Received	1		
Further Information Received on <u>14/12/2022</u>	Nature	The applicant provided clarifications on the development parameters and operational details of the proposed development, enclosing replacement pages of the application form and drainage assessment, as well as revised layout plans.	
	Applicant's Submission	No softcopy provided by Applicant. Hardcopies available at Planning Enquiry Counters for inspection.	
	Decision	Accepted and Exempted from Publication	
Application Available for Public Inspection	02/02/2022		
Until (tentative date of meeting)	03/02/2023		
Applications at the Site/Building	[press here]		
Remark	Deferred on 09/12/2022		